

Strain Energy - Stretching

$$U = \frac{1}{2} \sigma \epsilon \Delta V$$

① load controlled / force controlled
- apply $P \rightarrow$ measure ϵ

② displacement controlled
- apply $\epsilon \rightarrow$ measure P

① load controlled

$$\sigma_{xx} = \frac{P}{A}$$

$$\sigma_{ij} = \begin{bmatrix} \sigma_{xx} & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 0 & 0 \end{bmatrix} \quad \epsilon_{ij} = \begin{bmatrix} \epsilon_{xx} & 0 & 0 \\ 0 & -\nu \epsilon_{xx} & 0 \\ 0 & 0 & -\nu \epsilon_{xx} \end{bmatrix}$$

$$\epsilon_{ij} = ? \rightarrow \epsilon_{ij} = \frac{\sigma_{ij}}{E}$$

② displacement controlled

$$\sigma_{ij} = ? \rightarrow \sigma_{ij} = E \epsilon_{ij}$$

$$\epsilon_{xx} = \frac{\Delta L}{L} \quad \epsilon_{ij} = \begin{bmatrix} \epsilon_{xx} & 0 & 0 \\ 0 & 0 & 0 \\ 0 & 0 & 0 \end{bmatrix}$$

$$U = \frac{1}{2} \int_V \sigma_{ij} \epsilon_{ij} dV = \frac{1}{2} \int_V \sigma_{xx} \epsilon_{xx} dV = \frac{1}{2} \int_V \frac{P^2}{EA^2} dV$$

$$U = \frac{1}{2} \int_{-L/2}^{L/2} \int_{-b/2}^{b/2} \int_{-b/2}^{b/2} \frac{P^2}{EA^2} dx dy dz = \frac{1}{2} \frac{P^2}{EA} \int_{-L/2}^{L/2} dx$$

$$U_s = \frac{1}{2} \int_0^L \frac{P^2}{EA} dx$$

$$U = \frac{1}{2} \int_V (E \epsilon_{ij}) \epsilon_{ij} dV = \frac{1}{2} \int_V E \epsilon_{ij}^2 dV$$

$$U_s = \frac{1}{2} E A \int_0^L \epsilon^2 dx$$

Stretching Energy

$$U_s = \frac{1}{2} EA \int_0^L \epsilon^2 dx$$

$\rightarrow$ equivalent to strain energy density U_s

Solving

$$U_s \sim EA \epsilon^2 L$$

$$U_s \sim \frac{1}{EA} P^2 L$$

$\int_0^L dx$

Strain Energy - Bending

① Beam: pure bending $\rightarrow$ applied moment

$$\sigma_{xx} = -\frac{M_y}{I_z} y$$

$$U_b = \frac{1}{2} \int_V \sigma_{xx}^2 dV = \frac{1}{2} \int_V \left(\frac{M_y^2}{I_z^2} y^2 \right) dV = \frac{1}{2} \int_V \frac{M_y^2}{I_z^2} y^2 dV$$

$$U_b = \frac{1}{2} \int_0^L \frac{M_y^2}{I_z^2} \int_{-b/2}^{b/2} y^2 dy dx \rightarrow U_b = \frac{1}{2} \int_0^L \frac{M_y^2}{EI_z} dx$$

② Beam: pure bending $\rightarrow$ applied curvature

constitutive relationship: $M = K EI$

$$M = K EI$$

$$U_b = \frac{1}{2} \int_0^L \frac{(K EI)^2}{EI} dx \rightarrow U_b = \frac{EI}{2} \int_0^L K^2 dx$$



$$K = \frac{1}{R}$$

$$I = \frac{1}{12} b^3$$

$$U_b \sim E h^3 b K^2 L$$

Strain Energy - Torsion

$$\sigma_{xx} \sim \frac{M_y}{I_z}$$

$$\sigma_{xy} \sim \frac{T \rho}{J}$$

bending moments $\neq$ torsion $\Rightarrow$ couple

$$T = \tau \rho J$$

moment at twist $\neq$ shear stress

purely elastic

$$\sigma = E \epsilon$$

$$\sigma = G \gamma$$

shear modulus

$$U_t = \frac{GJ}{2} \int_0^L \tau^2 dx$$



Stretching & Bending

$$U_s \sim E h b \int_0^L \epsilon^2 dx \sim E h b L \epsilon^2$$

$$U_b \sim E h^3 b \int_0^L K^2 dx \sim E h^3 b L K^2$$

$$\frac{U_b}{U_s} \sim \frac{E h^3 b L K^2}{E h b L \epsilon^2} \sim h^2 \left(\frac{K}{\epsilon} \right)^2$$

$\rightarrow$ thin objects are easy to bend and hard to stretch

① let's assume our beam is inextensible

$$U_s = 0$$

② then objects are stiff, but prone to instability