



Pan-American Advanced Studies Institute
**The Science of Predicting and Understanding Tsunamis, Storm
Surges and Tidal Phenomena**

Boston University – Mechanical Engineering

**NONLINEAR PROPAGATION OF STORM WAVES:
INFRAGRAVITY WAVE GENERATION AND DYNAMICS**

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Associated National Universities



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PRESENTATION OUTLINE

I. Motivation

II. Nonlinear Wave Propagation in the Nearshore

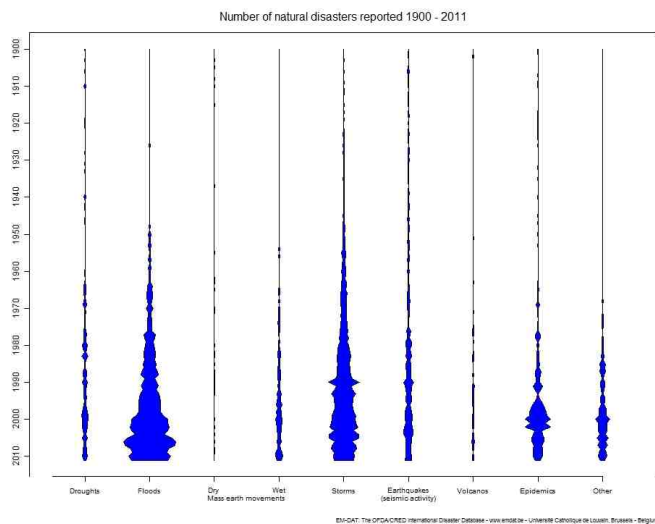
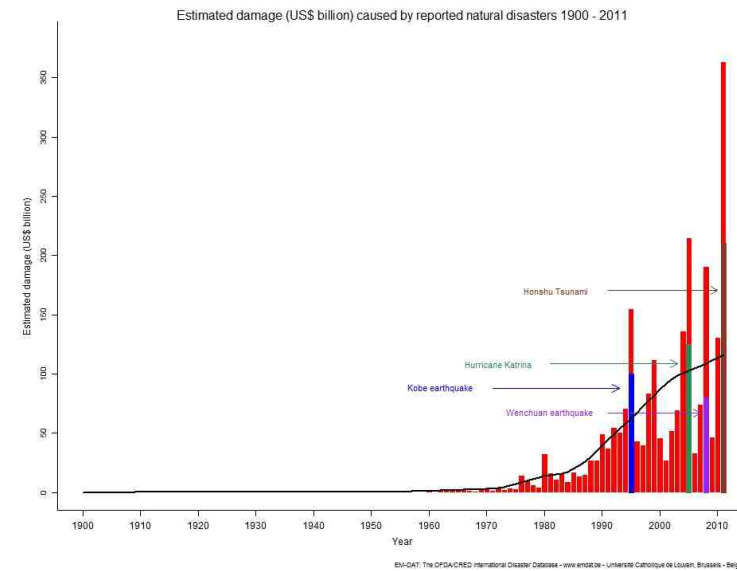
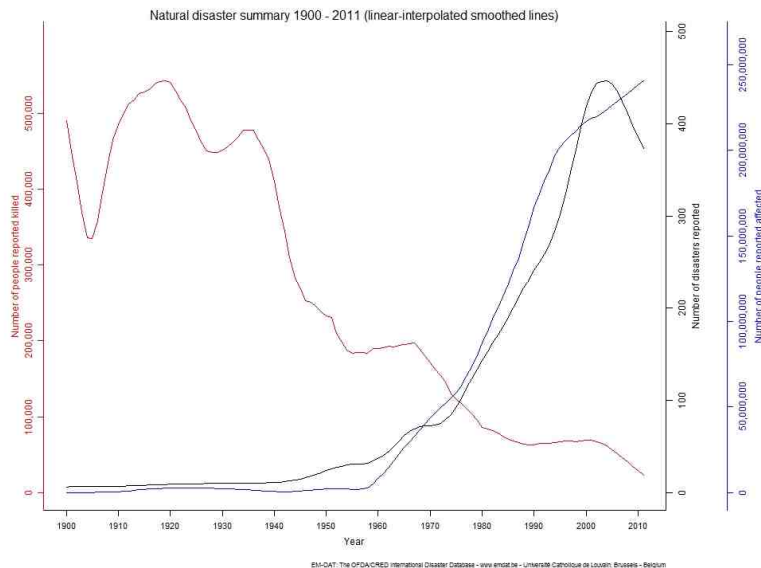
III. A Numerical and Experimental Study on Infragravity Waves

IV. Chile as an International Natural Coastal Laboratory

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- The number of ND have skyrocketed over the last century
- The number of affected people and estimated damage follow the trend
- The number of dead people decouples from the trend

Source : EM-DAT International Disasters Database
CRED - <http://www.emdat.be/>

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- **Extreme, large and rapid flooding**
 - Tsunamis, flash floods, GLOF, storm waves
- **Significant consequences**
 - Human lives
 - Facilities and structures
 - Social and environmental impact



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Recent Large River Floods in Chile



Talca, 2008



Chaitén, 2008



BíoBío, 2006



Licantén, 2008

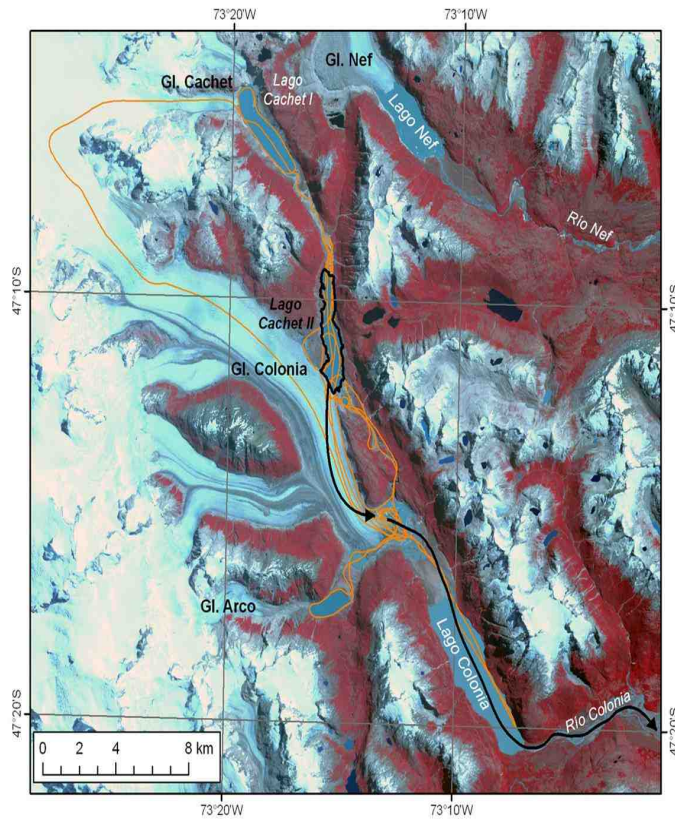


Mapocho, 1982

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Recent Glacial Lake Outburst Floods in Chile

- GLOF in the Chilean Patagonia (Chachet 2)
- More than 6 episodes since april 2008
- 4.5m stage rise and 3500m³/s peak discharge in Baker River in less than 48 hours.



April 2008



May 2008

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Recent Major Tsunamis in Chile



Valdivia, 1960



Constitución, 2010



Pelluhue, 2010

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Recent Major Storm Waves in Chile



Source : El Mostrador – <http://www.elmostrador.cl/>
August 16th 2012

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Nonlinear waves are in permanent action, shaping our environment, and questioning the way the society fits in it



Truc vert beach, French Atlantic Coast
Courtesy of P. Larroudé (LEGI)

- **Wide temporal and spatial scales**
- **Complex nonlinear interactions**
- **Strange consequences?**

⇒ Amazing research opportunities for contributing to the understanding of these processes and their consequences



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II. NONLINEAR WAVE PROPAGATION

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**Shoaling,
breaking and
run-up of an
incident
laboratory
wave**

**⇒ “Simplified” 2DV
view of nearshore
wave propagation**

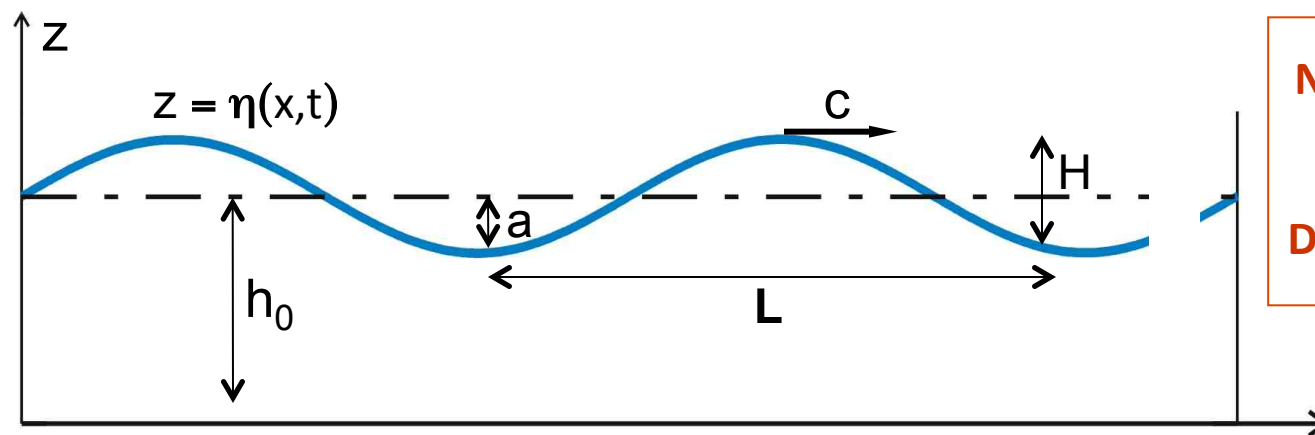
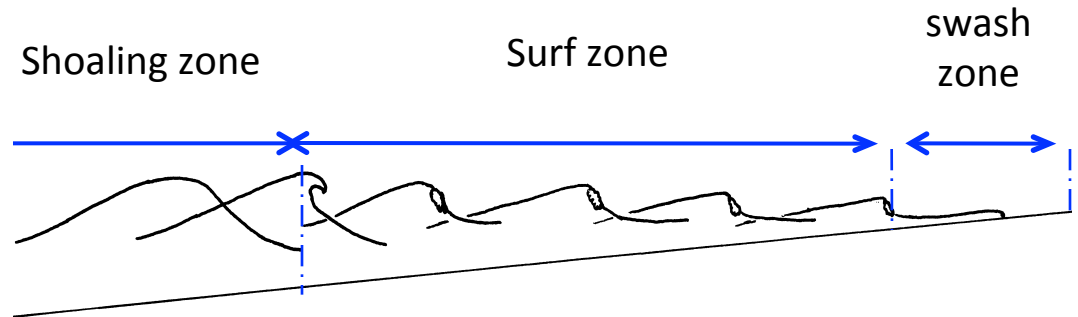


Source : Laboratory Experiment in the LEGI Wave Flume, France

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Nonlinearity $\varepsilon = a / h_0$

Dispersion $\sigma = h_0 / L$

Long wave (shallow water) models $\sigma \leq 0.1$

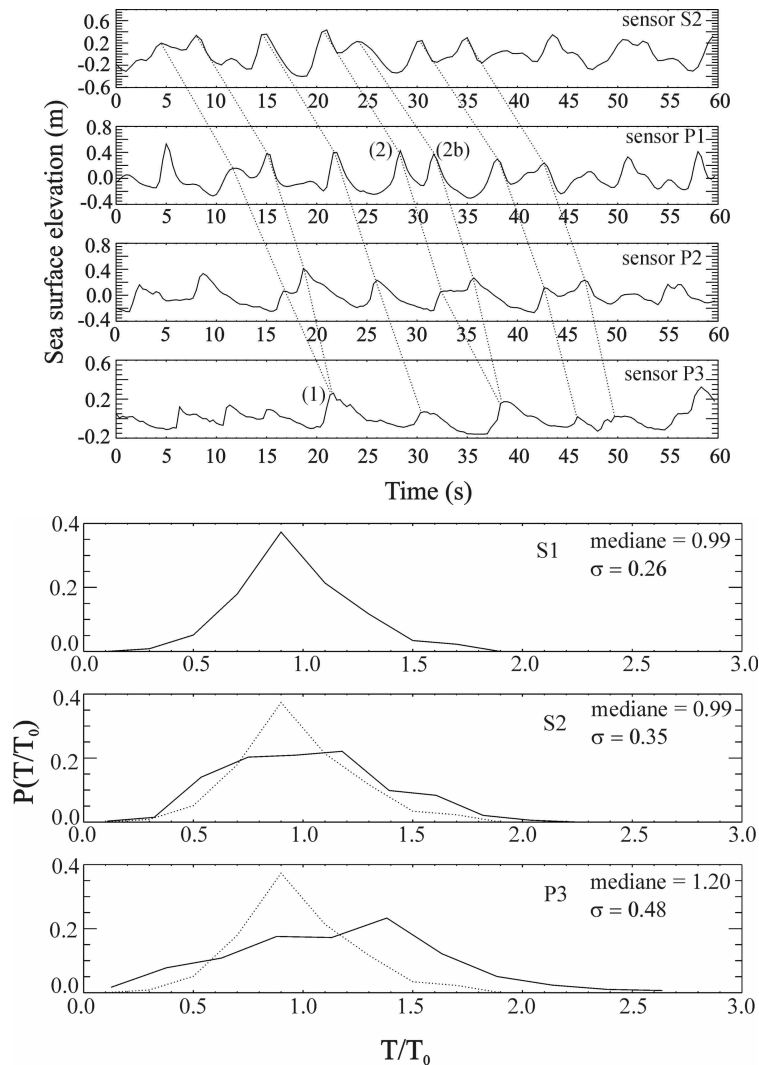
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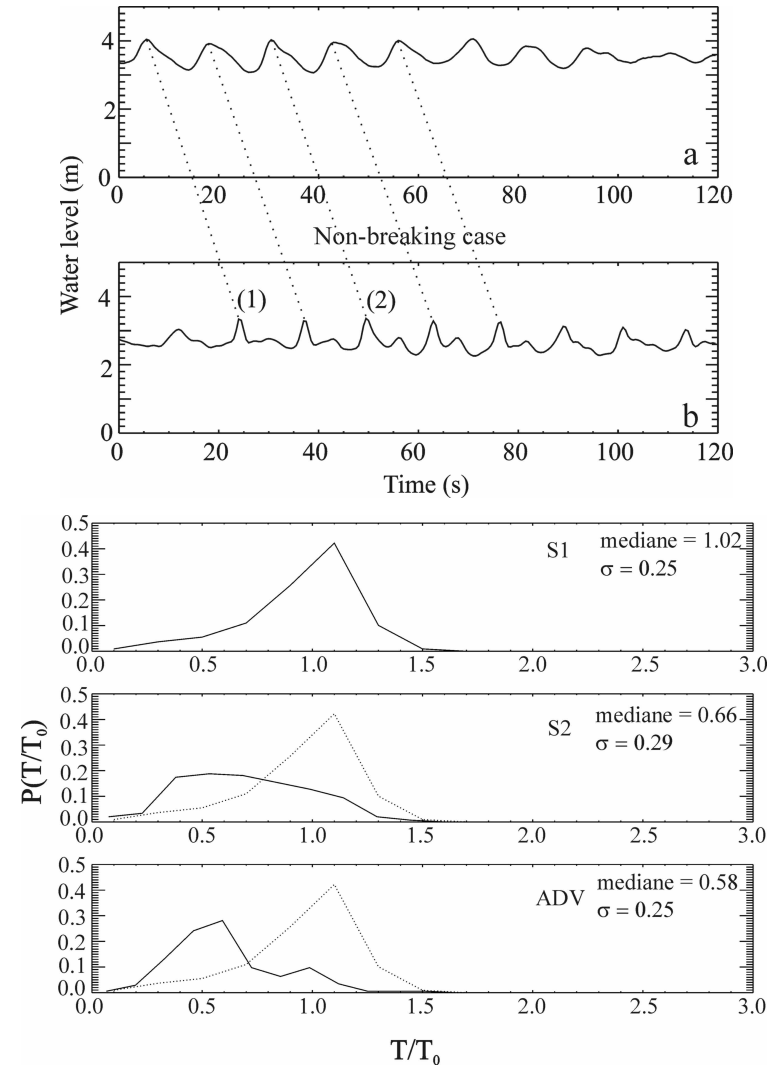
Wave merging in the inner surf zone

Sénéchal, Dupuis, Bonneton (2001)



Higher harmonics release after a bar

Sénéchal, Bonneton, Dupuis (2002)



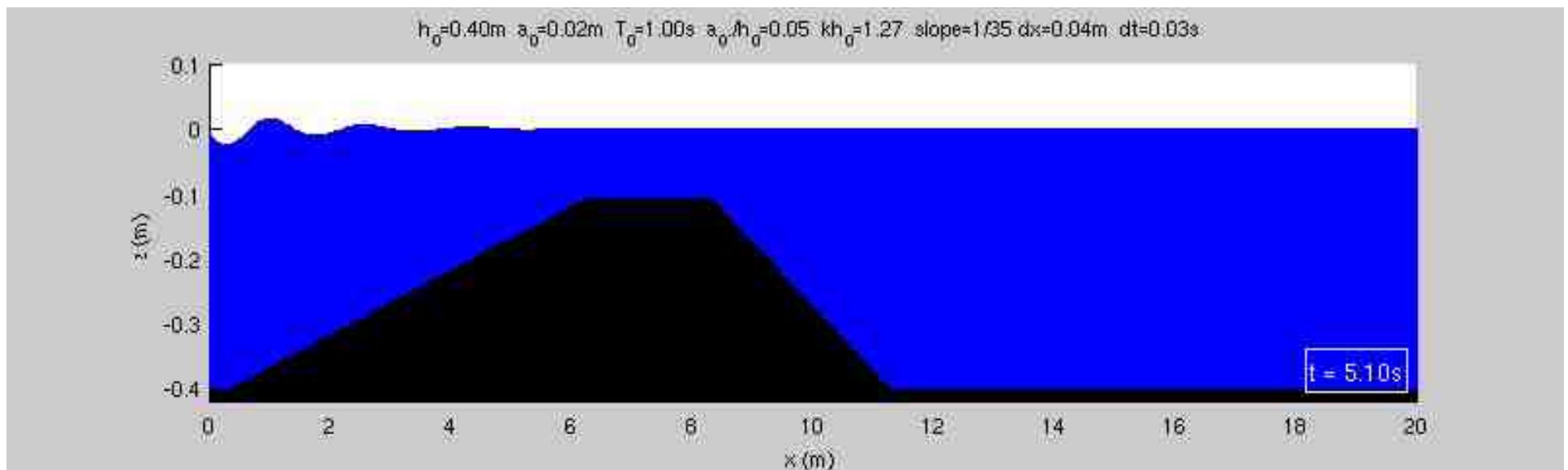
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Regular waves shoaling over a submerged bar, and higher harmonics release once water depth increases

- ⇒ Generation of bound higher harmonics in the shoal (shallow water propagation)
- ⇒ Over “intermediate” depths, higher harmonics travel at their “own” speed

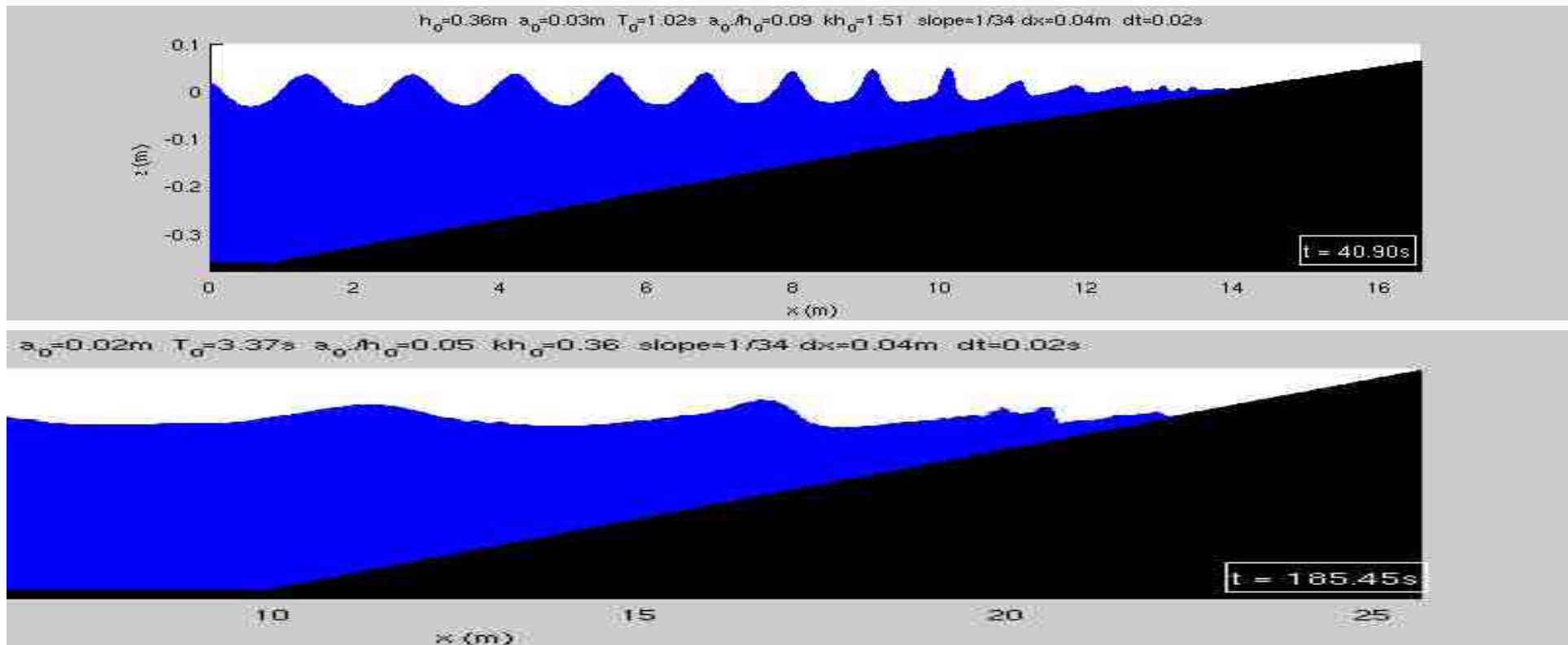


Source : Cienfuegos et al., 2007 ; Dingemans, 1994 experiments

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- ⇒ Wave propagation creates a set-down and set-up in the mean sea level
- ⇒ Fission of the main wave front for longer period waves

Source : Cienfuegos et al., 2010 ; Hansen and Svendsen, 1979 experiments

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Tsunami wave fission in the Sendai area, Japan 2011



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Set-down/Set-up over a uniform beach under regular wave propagation

⇒ **Radiation stress concept for wave-averaged quantities (Longuet-Higgins and Stewart, 1962)**

$$\frac{\partial S_{xx}}{\partial x} + \rho g (\bar{\eta} + h) \frac{\partial \bar{\eta}}{\partial x} = 0$$

$$S_{xx} = E \left\{ \left[\cos^2 \theta + 1 \right] \frac{C_g}{C} - \frac{1}{2} \right\}$$

Source : Bowen et al., 1968

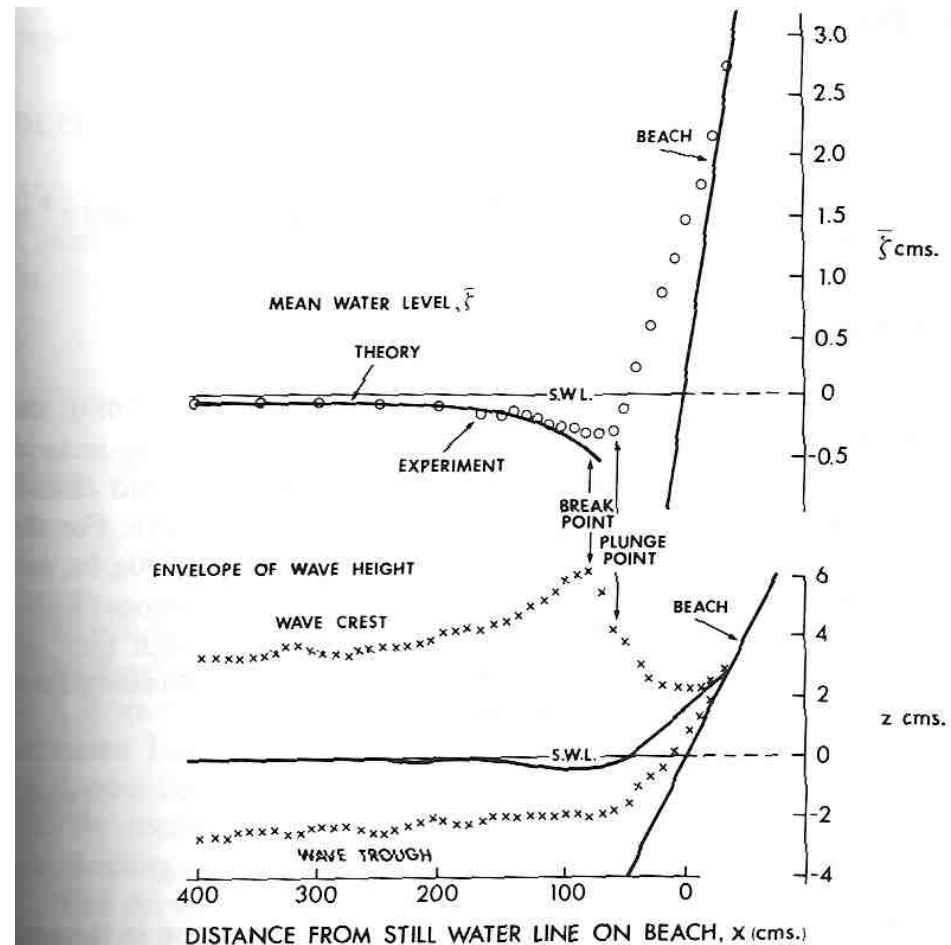


Figure 5.1 Comparison of experiments with theory for set-down and set-up on a plane beach. Data: wave period = 1.14 s; deep water wave height $H_{\infty} = 6.45$ cm; breaker height $H_b = 8.55$ m; beach slope = 0.082 (from Bowen et al., 1968, *J. Geophys. Res.*).

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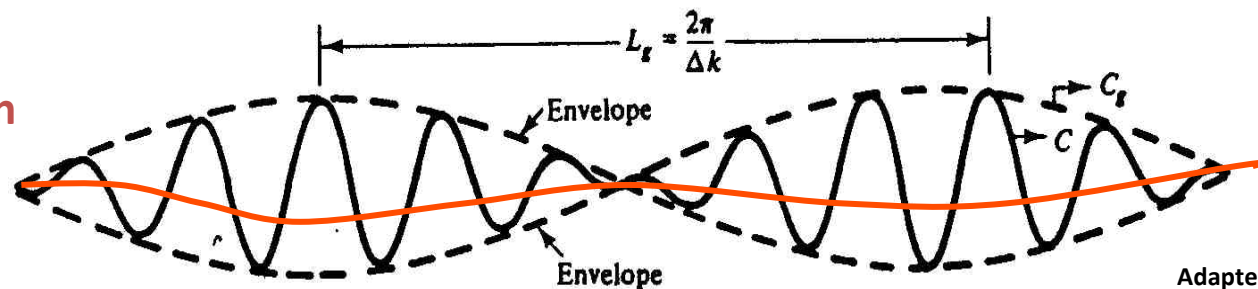
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Infragravity wave (IG) generation under random wave forcing

⇒ Long waves forced by high-frequency wave groups

2nd Order Long Wave Generation Mechanism

Negative correlation
between group
envelope and IG
waves



Adapted from Dean and Dalrymple, 1984

Figure 4.12 Characteristics of a “group” of waves.

Mean water level pulsation associated to wave groups' set-up/set-down (Longuet-Higgins and Stewart, 1962)

⇒ Bound long waves propagating at group velocity

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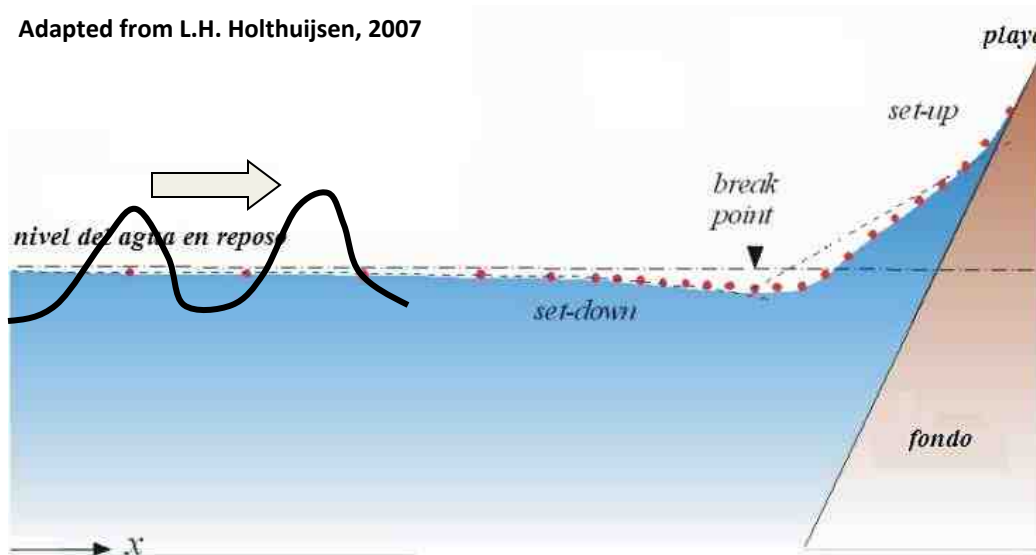
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Infragravity wave (IG) generation under random wave forcing

⇒ Long waves forced by a variable breakpoint location

The slow modulation of the breaking point under a random wave field produces a dynamic set-up/set-down

Adapted from L.H. Holthuijsen, 2007



Positive correlation
between group envelope
and IG waves

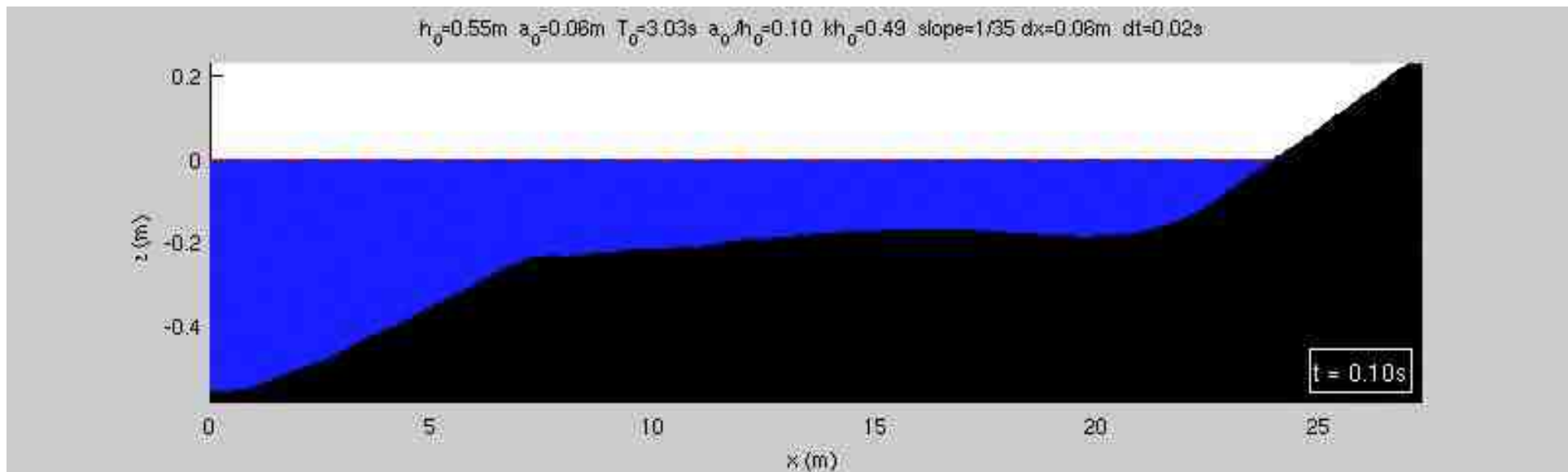
SURF BEAT (Symonds, 1982)

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Practical consequences of IG waves: Increased run-ups



⇒ Random short waves transfer energy to infragravity waves while propagating towards the shore

⇒ Very important for swash zone dynamics and maximum observed run-ups

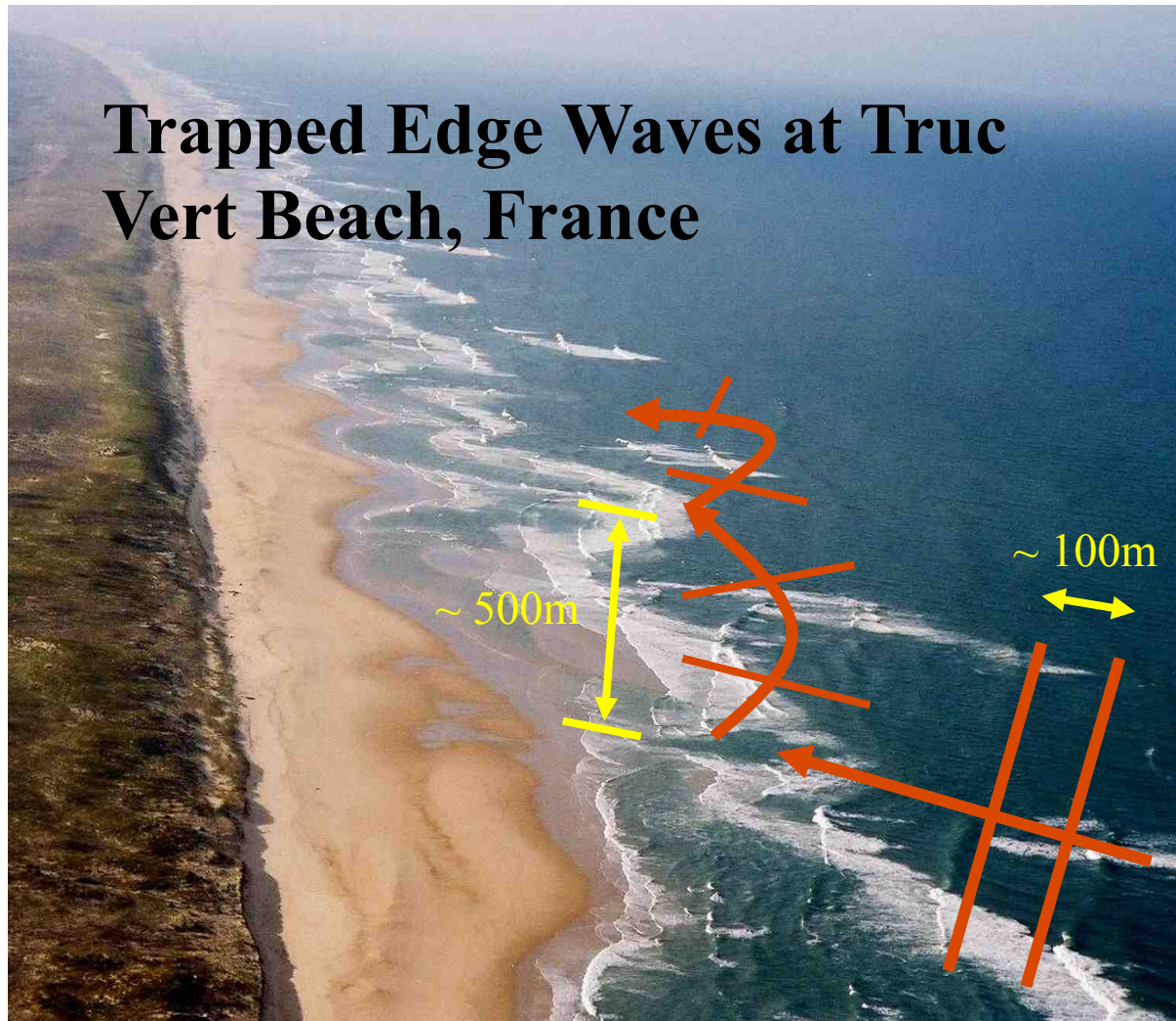
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Practical consequences of IG waves: Edge waves

Trapped Edge Waves at Truc Vert Beach, France



- The breaking process is responsible for the release of IG waves

- IG waves reflect at the coast (swash)

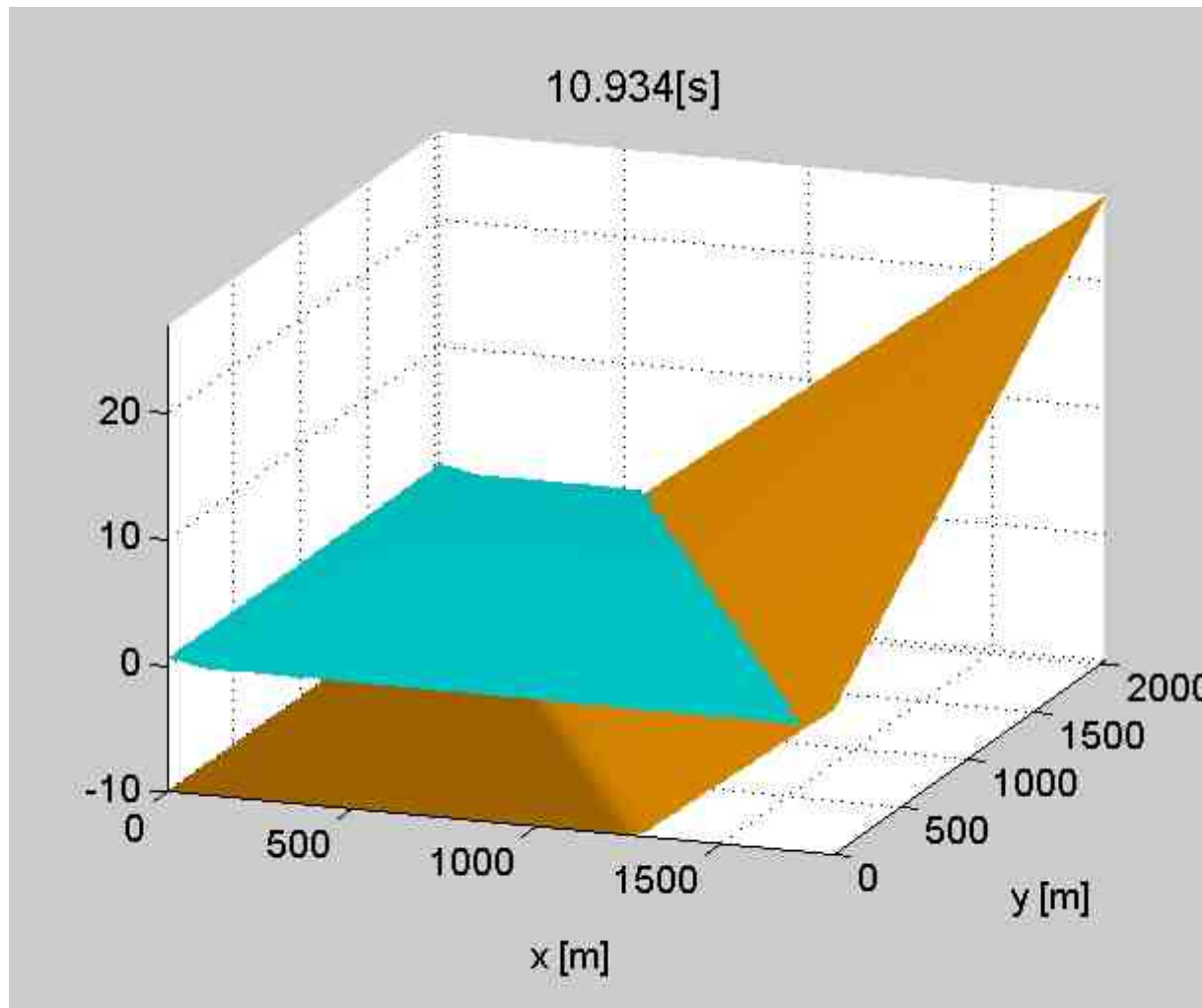
- Refraction traps IG (so-called edge waves)

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Practical consequences of IG waves: Edge waves



Incident periodic waves:
 $H=1\text{m}$; $T=100\text{s}$; $\theta_0=30^\circ$

Uniform beach profile:
 $h_0=10$; $s=1/40$

⇒ Reflected waves are trapped and propagate in the alongshore direction

⇒ Important consequences for maximum run-up distribution

NSWE FV model developed by M. Guerra (PUC MSc thesis)

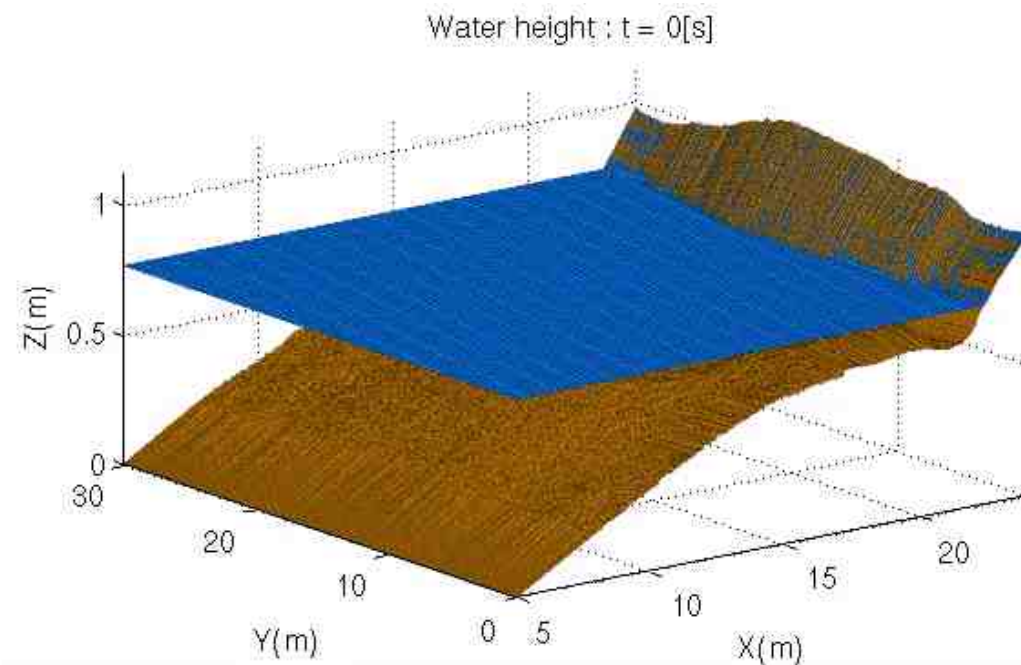
Numerical simulation by José Galaz (PUC undergrad student)

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Practical consequences of IG waves? Rip instabilities



- Frontal incident random wave trains with a small amplitude perturbation in the center
- Rip current generation
- Slow mean current pulsations
- Quasi-periodic ejection of drifters

PhD thesis of Leandro Suarez (PUC-LEGI grad student)
Suarez et al., ICCE 2012 ; Suarez et al., CD 2013





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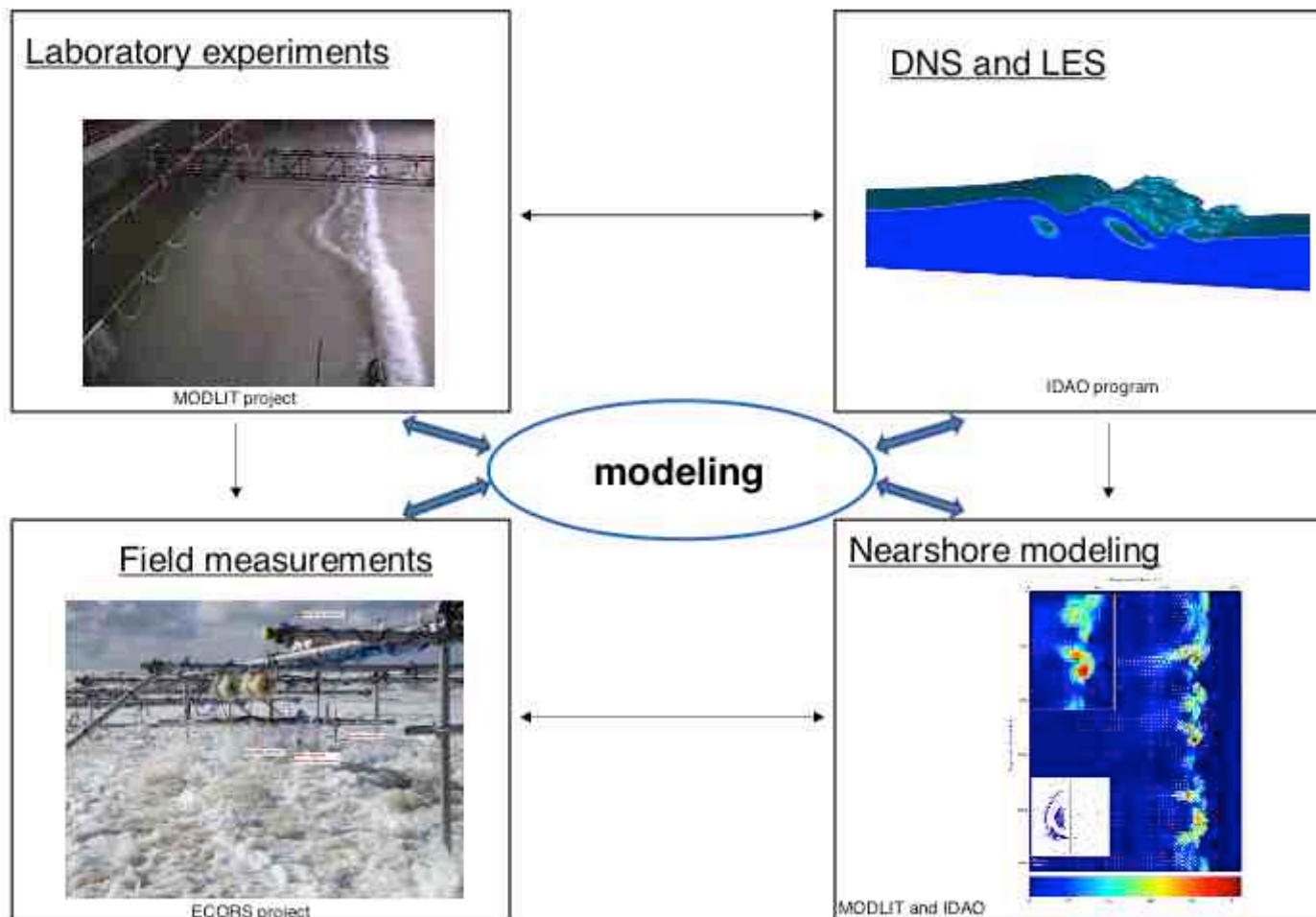
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Different methods to cope & understand water waves



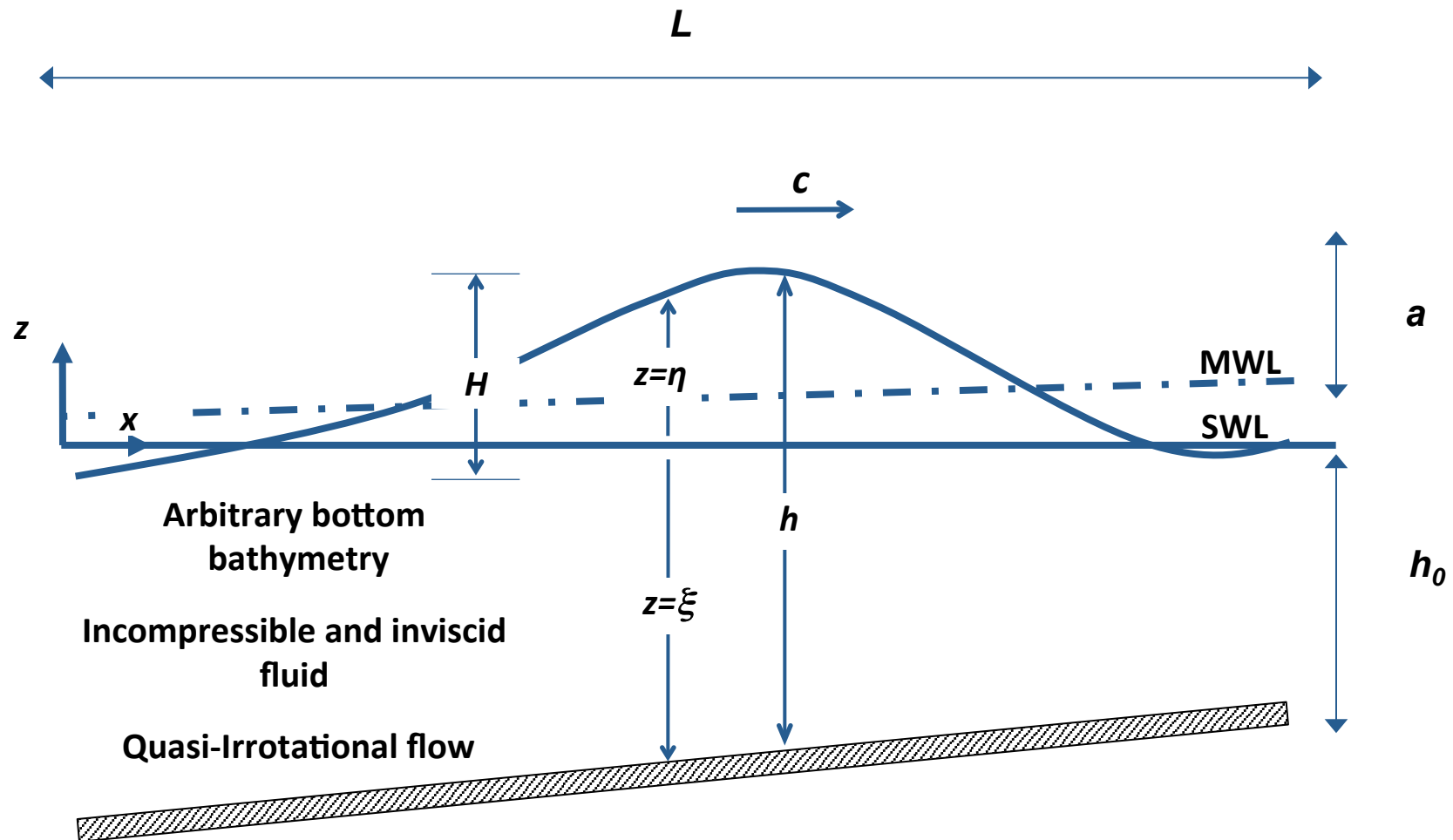
Source: P. Bonneton, 2010

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General Definitions and Framework



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Non-dimensional depth-averaged mass and momentum equations

$$h_t + \varepsilon [h\bar{u}]_x = 0$$

$$\varepsilon [h\bar{u}]_t + \left[h \left(\varepsilon \overline{u^2} + \bar{p} \right) \right]_x + \xi_x p|_{\xi} = 0$$

Nonlinearity $\varepsilon = a / h_0$

Dispersion $\sigma = h_0 / L$

Where the pressure field is written as :

$$p(x, z, t) = (\varepsilon \eta - z) + \varepsilon \sigma^2 \int_z^{\varepsilon \eta} \Gamma(x, z, t) dz$$

and the function Γ represents the vertical acceleration of fluid particles :

$$\Gamma(x, z, t) = \frac{Dw}{Dt} = w_t + \varepsilon \left(uw_x + \frac{1}{\sigma^2} ww_z \right)$$

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Using approximate expressions for the velocity field (potential flow theory) we get the fully nonlinear weakly dispersive Serre/Green-Naghdi Equations (Cienfuegos et al., 2006; Lannes and Bonneton, 2009)

$$h_t + \varepsilon[hu]_x = 0$$

$$(hu)_t + \varepsilon(hu^2)_x + h(h + \xi)_x + \sigma^2 \left\{ h^2 \left(\frac{1}{3} P(x, t) + \frac{1}{2} Q(x, t) \right) \right\}_x + \\ + \sigma^2 \xi_x h \left(\frac{1}{2} P(x, t) + Q(x, t) \right) = O(\sigma^4)$$

Where :

$$P(x, t) = -h(u_{xt} + \varepsilon uu_{xx} - \varepsilon u_x^2)$$

Dropping overbars for simplicity

$$Q(x, t) = \xi_x(u_t + \varepsilon uu_x) + \varepsilon \xi_{xx} u^2$$

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A 4th Order FV S/G-N Model (Cienfuegos et al., 2006, 2007)

- A strongly nonlinear and weakly dispersive Boussinesq system $O(\sigma^4)$

$$\begin{cases} h_t + F_x = 0 \\ q_t + G_x = 0 \end{cases}$$

With the following definitions for flux functions and auxiliary variables

$$F = hu$$

$$G = uq + g(h + \xi) - \frac{1}{2}u^2(1 + \xi_x^2) + hu_x\xi_x u - \frac{1}{2}(hu_x)^2$$

$$q = (1 + r)u - \frac{1}{3h}(h^3 u_x)_x$$

$$r = (h_x + \xi_x)\xi_x + \frac{1}{2}h\xi_{xx}$$

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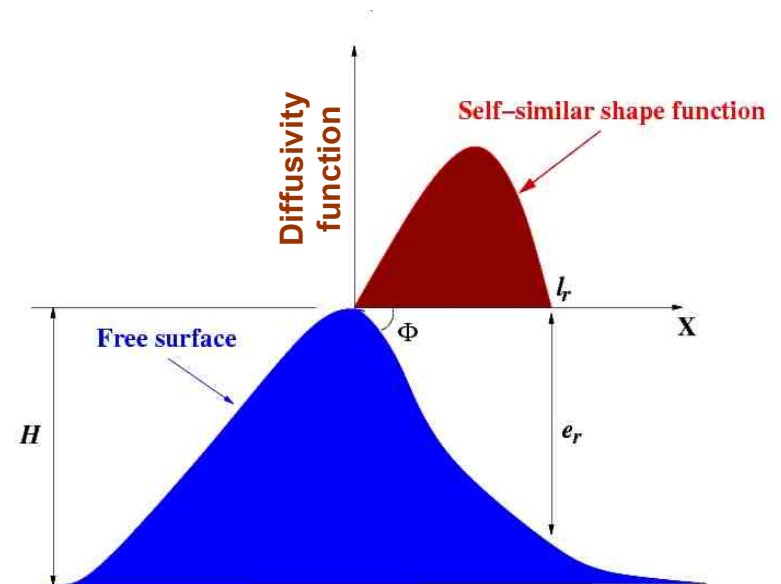
Incorporating Breaking Energy Dissipation (Mignot and Cienfuegos, 2009; Cienfuegos et al., 2010)

- Diffusive-like terms in both, mass and momentum conservation equations

$$\begin{cases} h_t + F_x + D_h = 0 \\ q_t + G_x + \frac{1}{h} D_{hu} = 0 \end{cases}$$

$$\begin{aligned} D_h &= (v_h h_x)_x \\ D_{hu} &= [v_{hu} (hu)_x]_x \end{aligned}$$

Local re-distribution of mass and momentum below breakers

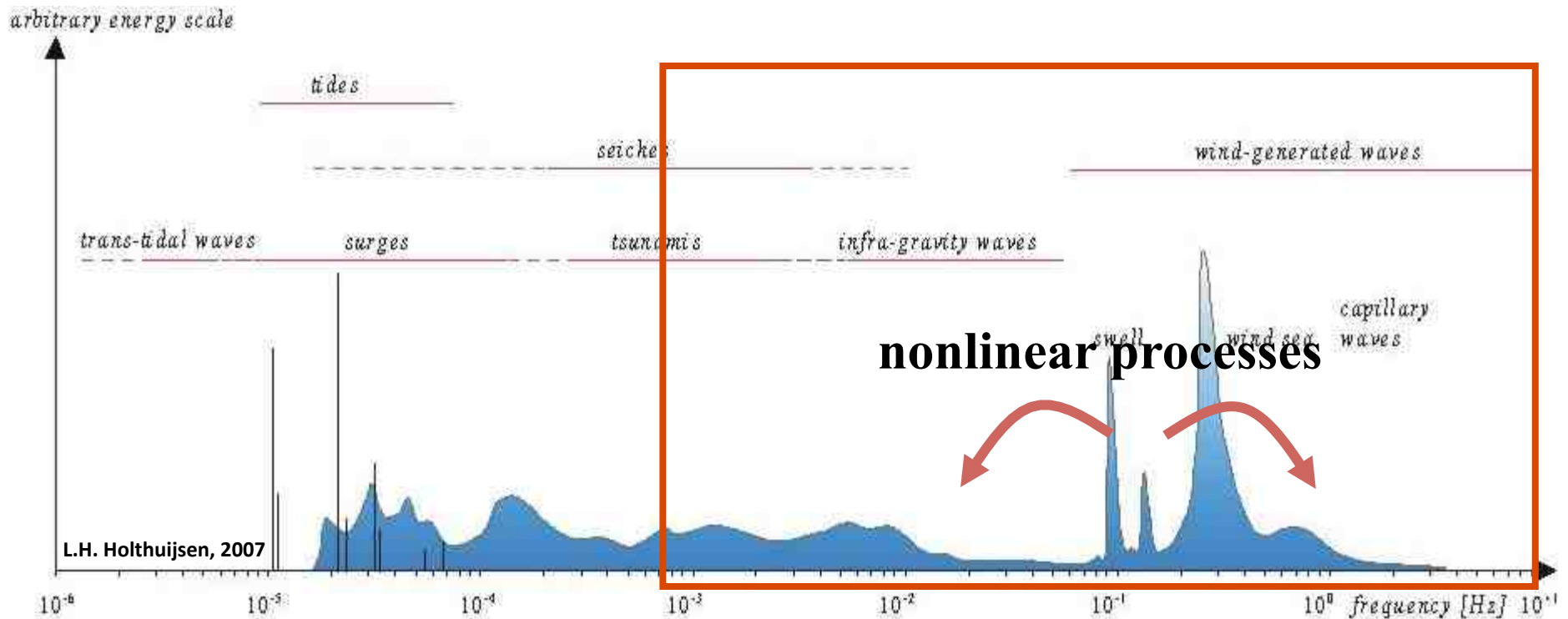


Similar to Kennedy et al., 2000

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- Typical wind-generated waves : 0.05Hz (20s) – 1Hz (1s)
- Nonlinear processes (shoaling, breaking, swash) generate infragravity (IG) waves : 0.001Hz (1000s) – 0.05Hz (20s)

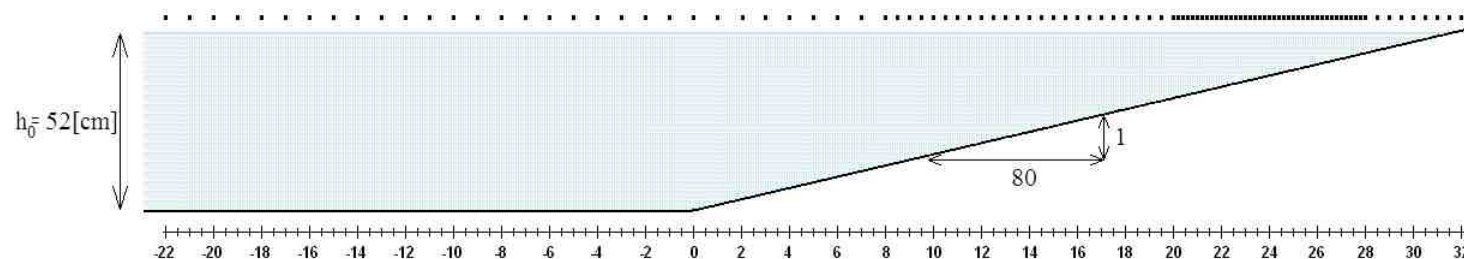
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Experimental Investigation at the Instituto Nacional de Hidráulica

- 70m long x 1.5m wide x 1.5m deep wave tank
- Piston wave paddle with active absorption (DHI)
- Resistive probes for free surface measurements
- Sontek MicroADV for velocity measurements
- Very mild slope beach 1/80



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Random Incident Wave Field

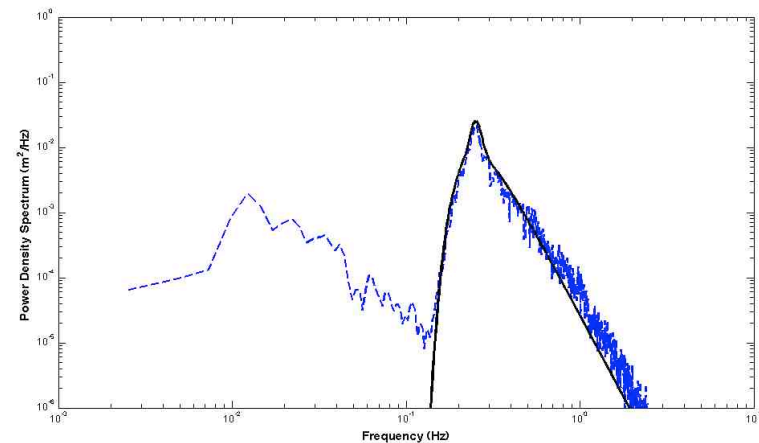
- Jonswap spectrum (Hasselmann, 1973) for incident wave field

$$T_p=4 \text{ s (0.25Hz)} ; H_{mo}=18 \text{ cm} ; h_0=52\text{cm}$$

- Without phase structure (uniform random distribution $[0,2\pi]$)

- Highly nonlinear shallow water regime

⇒ Although the forcing wave field does not contain energy in the infragravity band, experiments show important energy content at these frequencies



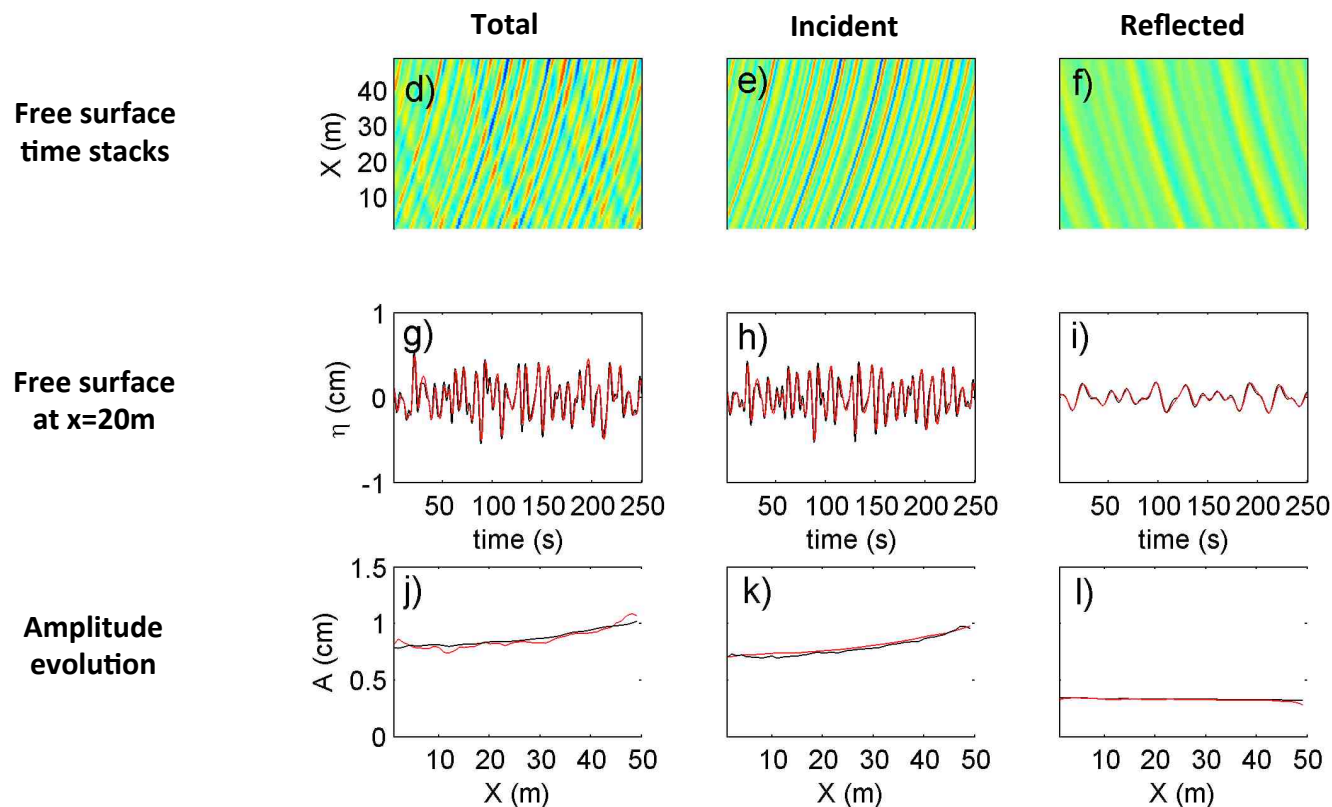
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Separation of Incident and Reflected Waves

- Radon transform (Deans, 1983) in time domain to separate incident and reflected signals (Almar et al., 2013)



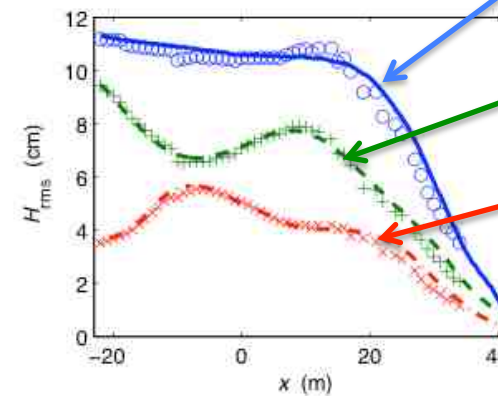
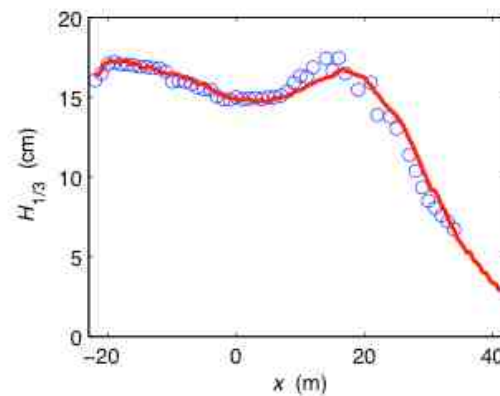
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Model Performance on High Frequency Waves

Wave height
and energy
distribution

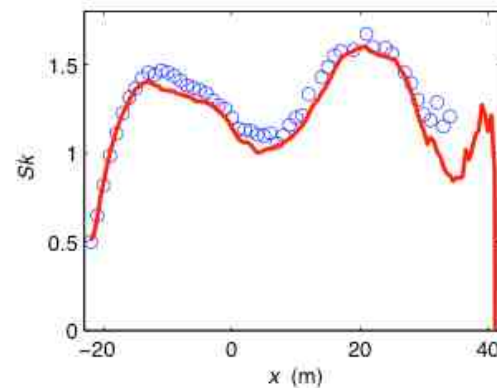


Total

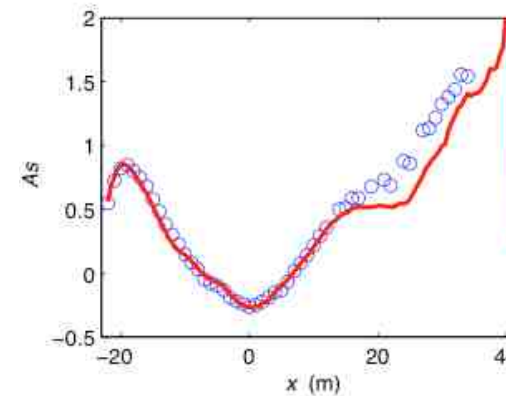
1st harmonic

2nd harmonic

Crest-trough wave
asymmetry (S_k)



Left-right wave
symmetry (A_s)

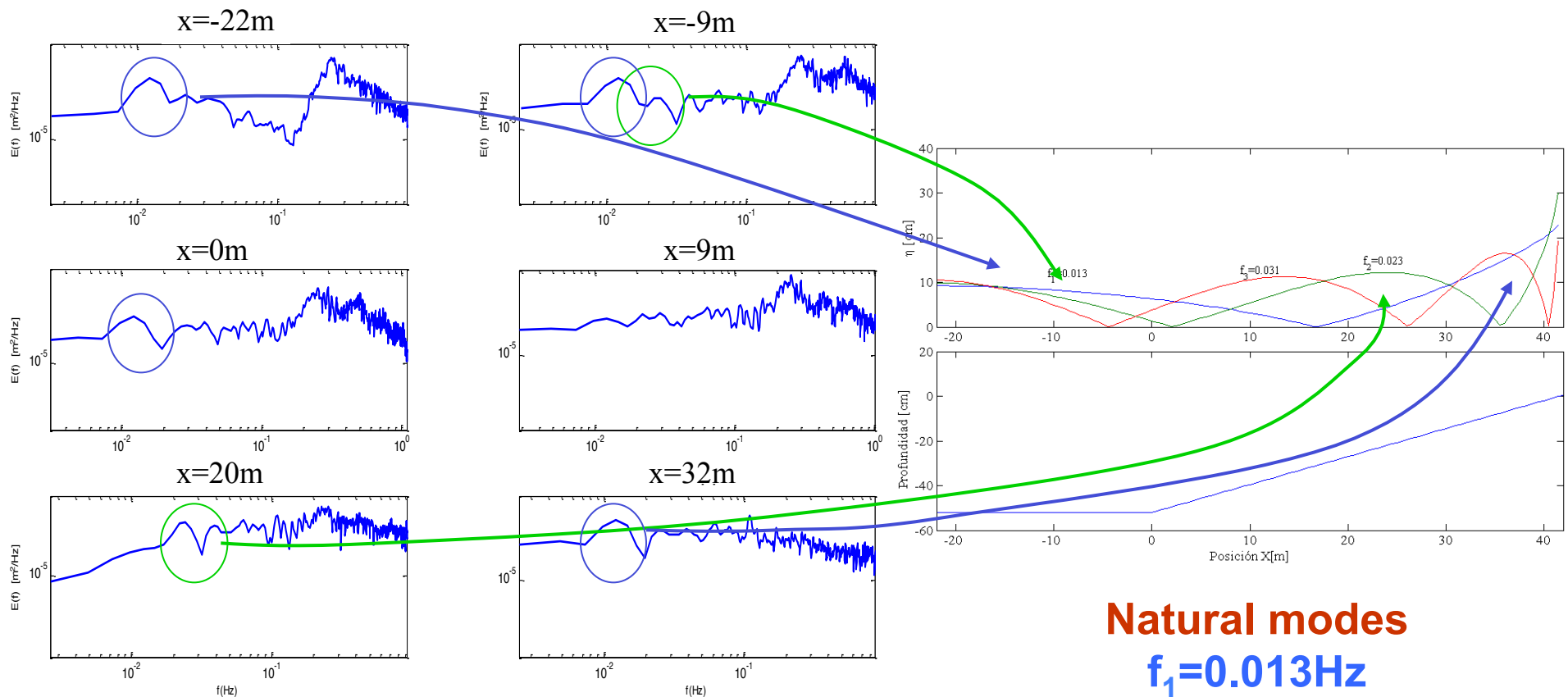


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Spectral Energy Evolution Along the Experimental Flume



Natural modes

$f_1 = 0.013\text{Hz}$

$f_2 = 0.023\text{Hz}$

$f_3 = 0.031\text{Hz}$

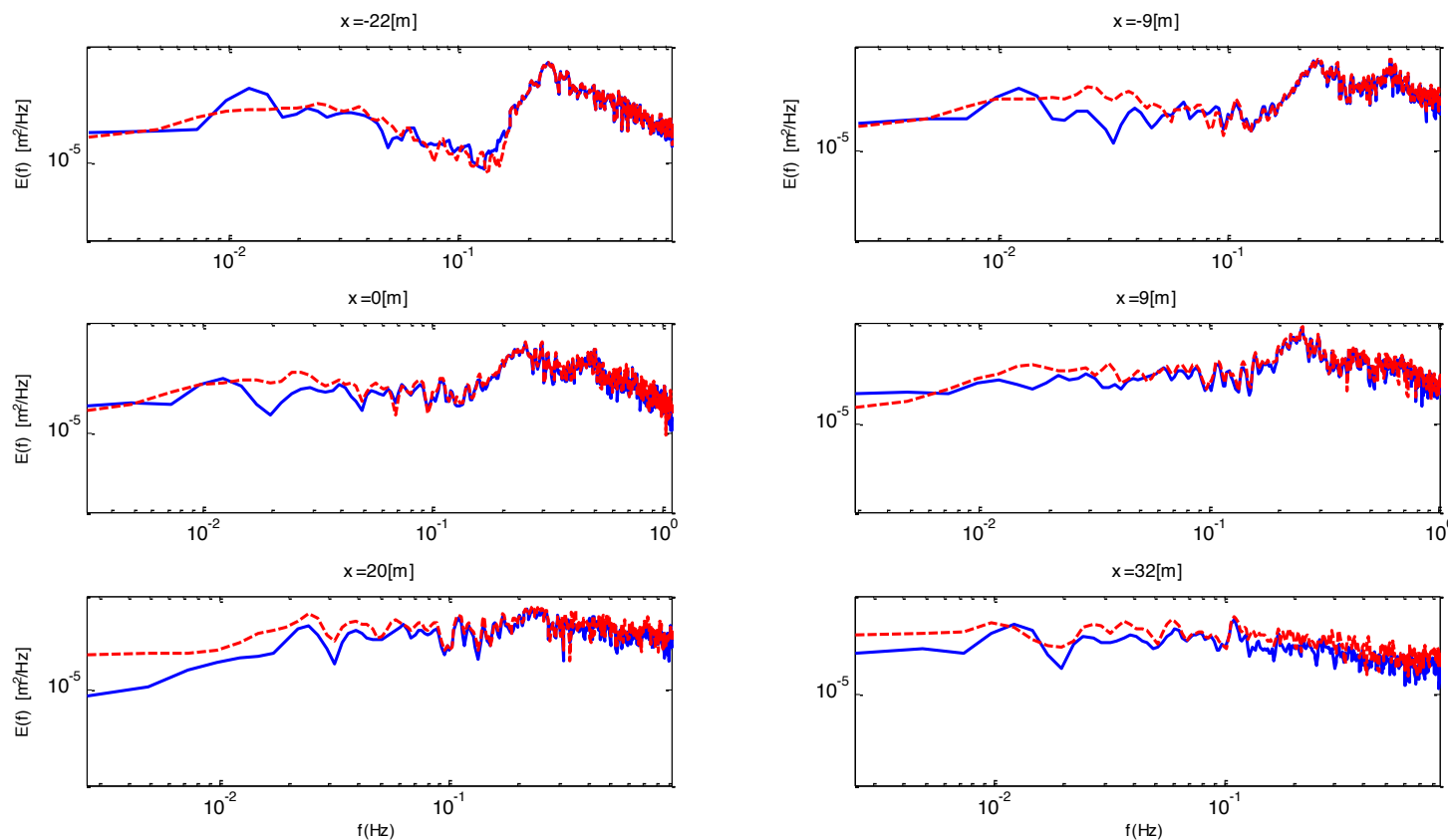
Partial reflection at the wave paddle produces low frequency selection in the flume

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Model Performance in Spectral Energy Evolution



Blue: Experiments
Red : Model

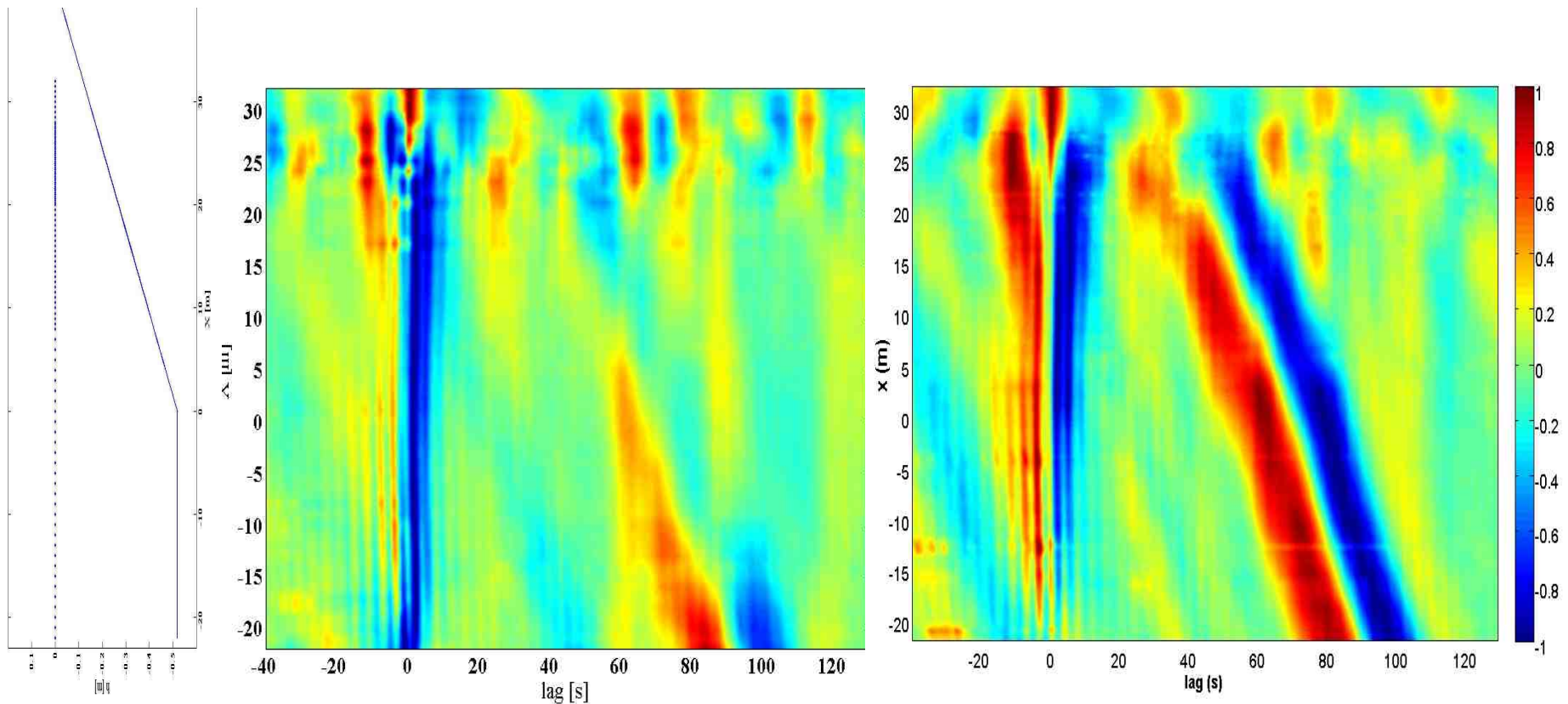
⇒ The model succeeds in transferring energy to higher harmonics and IG waves but no clear modal selection (open boundary condition)

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Cross-correlation maps between group amplitude and IG wave amplitude at each location



► Experimental

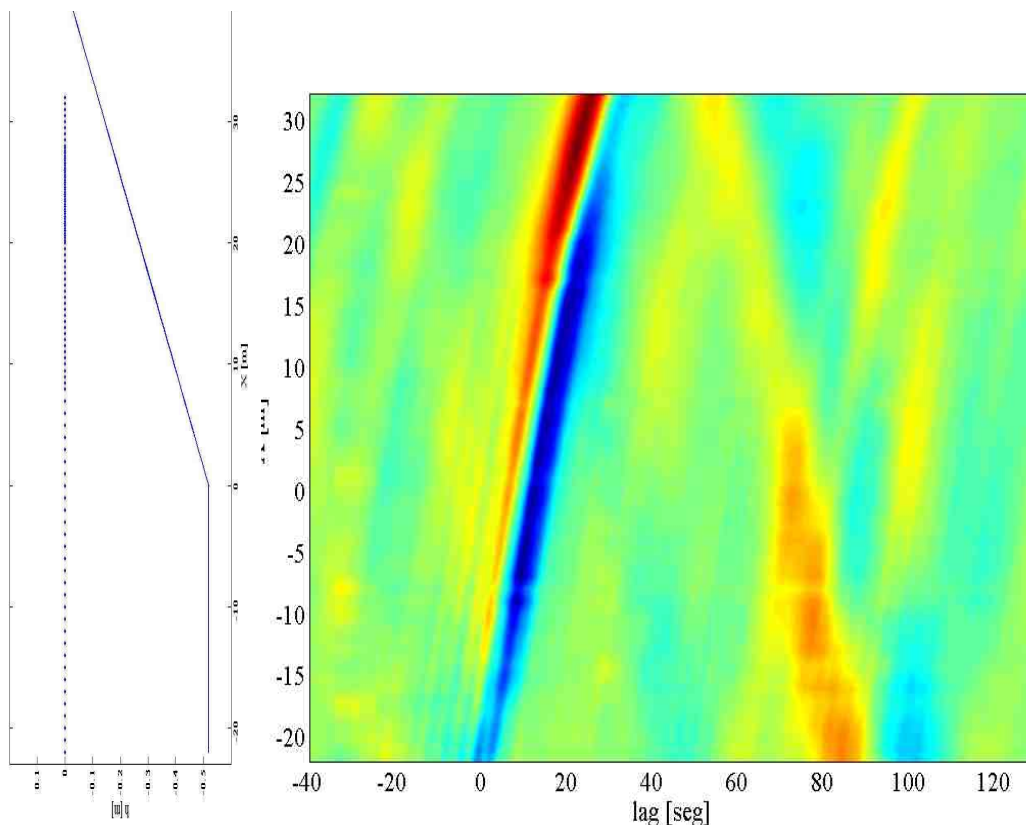
► Numerical

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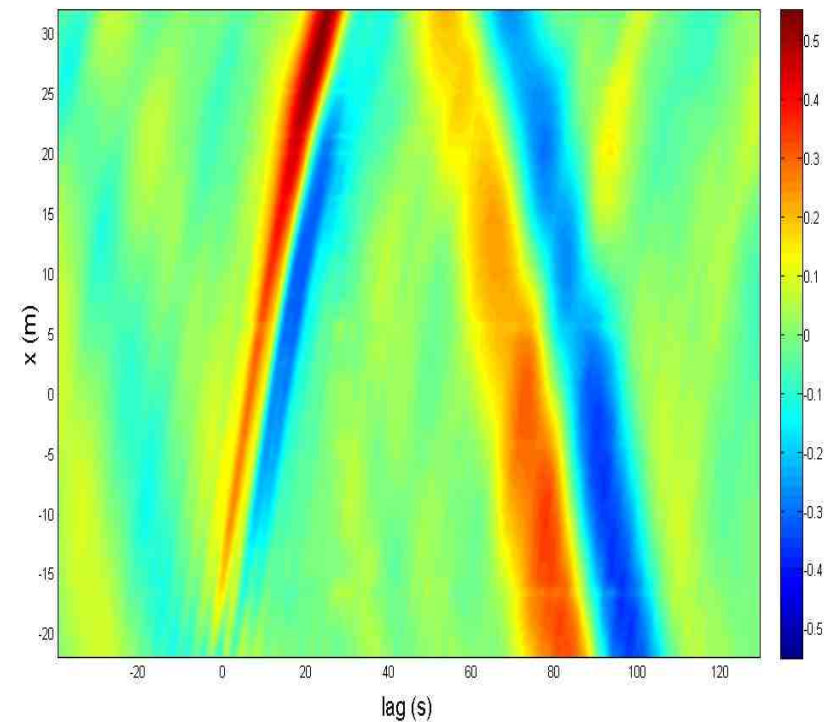
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Cross-correlation maps between group amplitude at the first gauge and IG wave amplitude at each location



► Experimental



► Numerical



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- A bound IG appears rapidly in the shoaling zone forced by short wave groups (negative correlation slightly lagging the group)
- A positive long wave surge preceding the group develops as the short waves propagate towards the shore (positive correlation running ahead of the group)
- There is a phase shift for the positively correlated IG wave in the inner surf zone
- Both IG waves appear to be partially reflected on run-up (some dissipation by breaking might occur)

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- The shallow water regime ($C_g \sim C$) produces a nearly resonant condition for IG wave forcing (Nielsen and Baldock, 2010)

⇒ The so-called N-shaped long wave is forced as a bound wave in the shoaling zone

Non-resonant condition

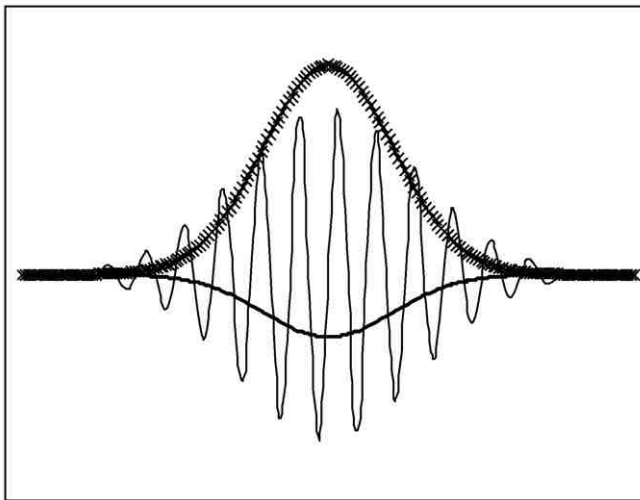


Fig. 1. Steady, bowl-shaped long wave (solid line) driven by a steady, non-resonant wave group (thin line) with S_{xx} varying as indicated by $-x-x$.

Resonant condition

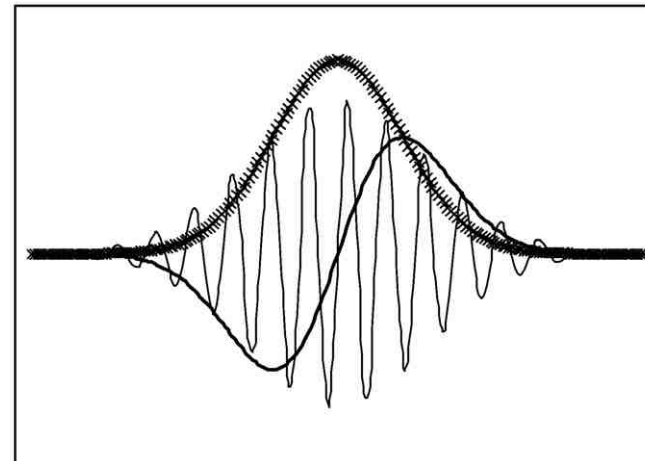


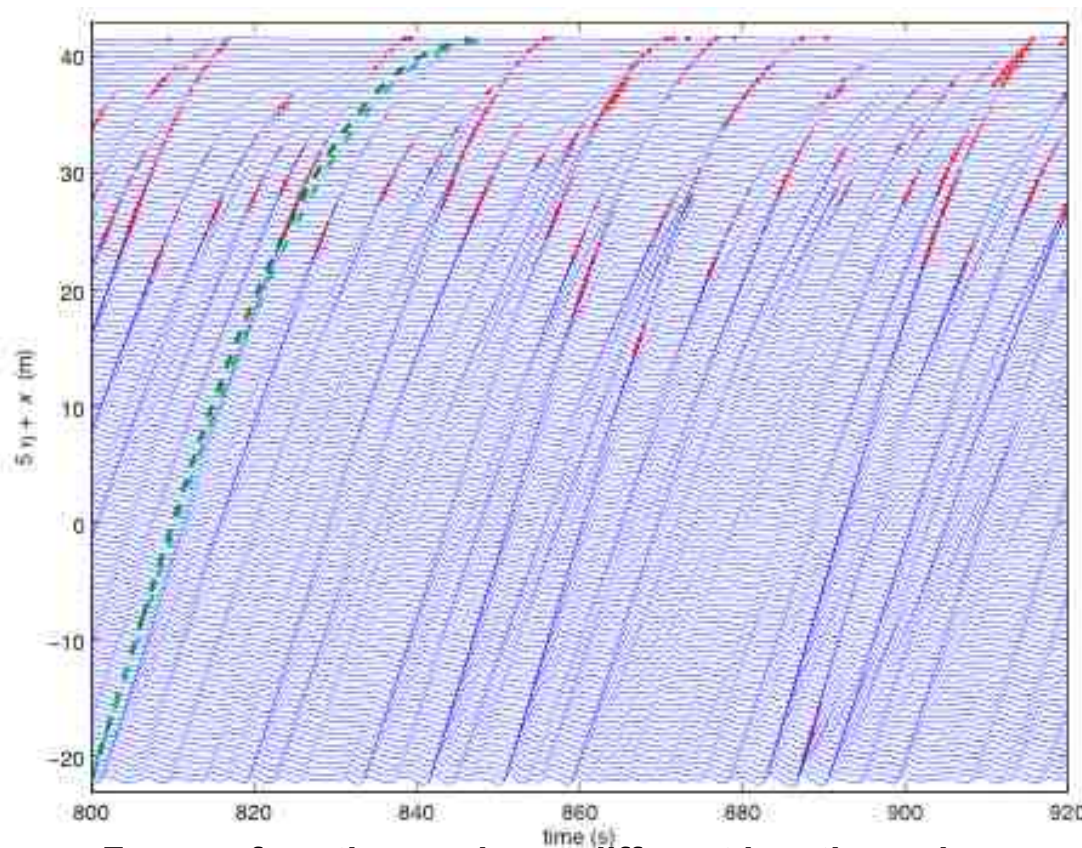
Fig. 3. At resonance, $c_g = \sqrt{gh}$, which happens in the shallow water limit, the Gaussian shaped S_{xx} forcing $-x-x$ from the wave pulse generates an N-shaped long wave which grows linearly with time.

III. INFRAGRAVITY WAVES

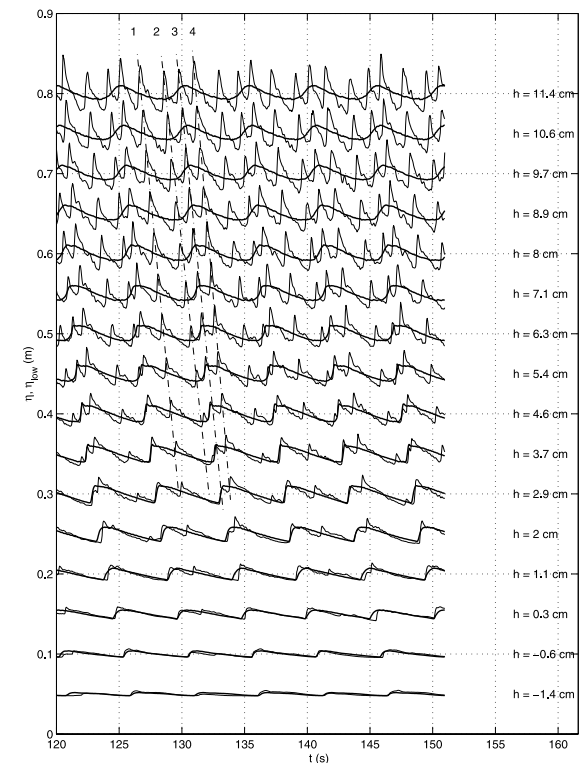
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- The phase shift in the positively correlated IG wave in the inner surf zone is due to a strong interaction between short waves and IG motions in very shallow waters (merging of short frequency waves)



Free surface time series at different locations along the numerical wave flume



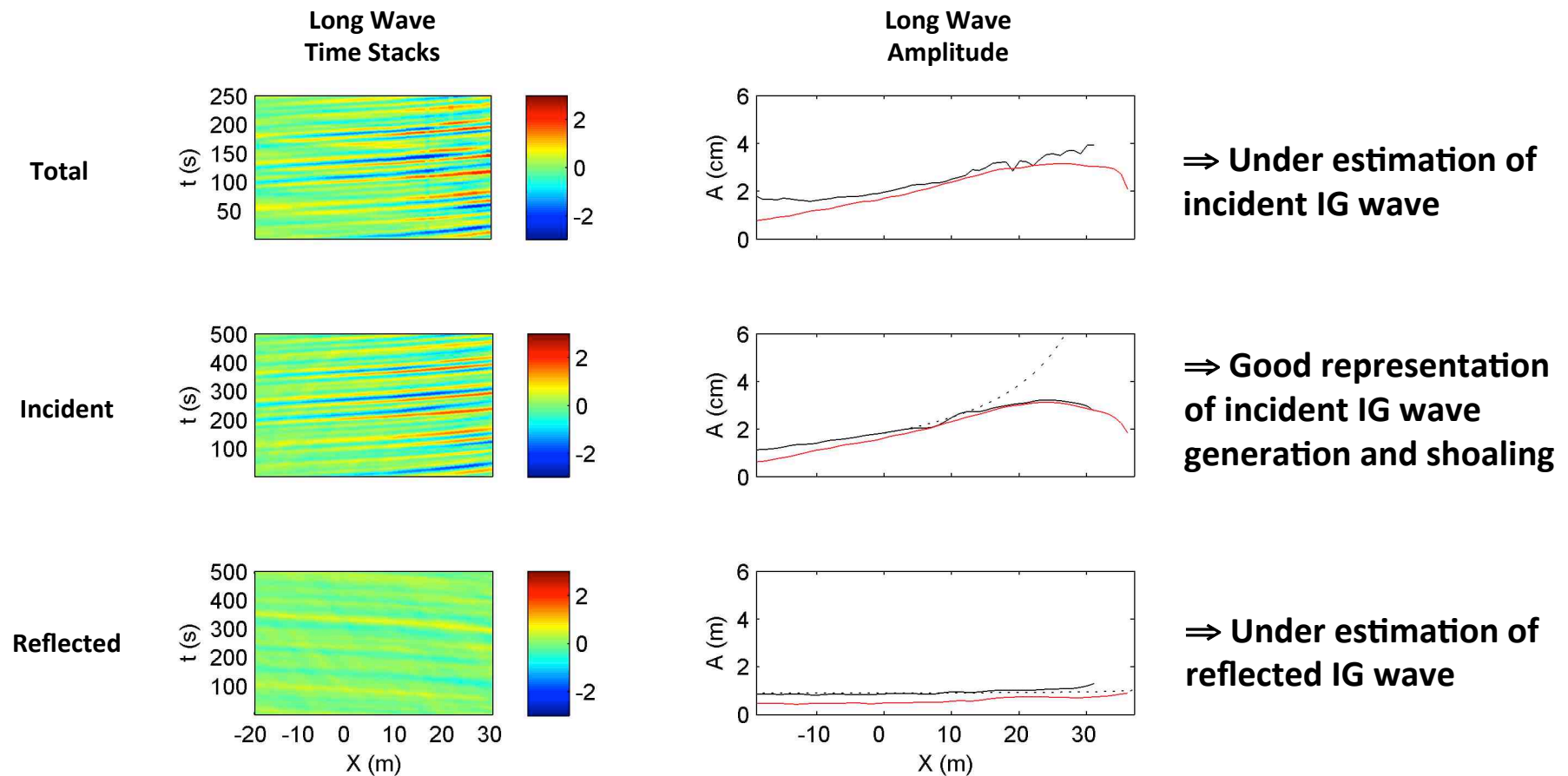
Merging of high frequency waves on a bi-chromatic experiment (Van Dongeren et al., 2007)

III. INFRAGRAVITY WAVES

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Model Performance for Incident and Reflected IG Waves



Black : Experiments

Red : Model

Dashed line : theoretical free long wave amplitude shoaling



III. INFRAGRAVITY WAVES

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Summary

- **Maximum run-ups under highly nonlinear waves are strongly dependent on infragravity wave motions (strong under estimation of maximum run-ups using linear wave theories)**
- **Fully nonlinear Boussinesq-type equations including breaking and run-up are able to cope with the complex energy transfer processes within the nearshore producing accurate estimations of short wave propagation and IG generation and dynamics**
- **More research is needed to (fully) understand nearshore wave processes and its (un)expected consequences on morphodynamics and coastal hazards**



Pan-American Advanced Studies Institute

The Science of Predicting and Understanding Tsunamis, Storm Surges and Tidal Phenomena

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PRESENTATION OUTLINE

I. Motivation

II. Nonlinear Wave Propagation in the Nearshore

III. A Numerical and Experimental Study on Infragravity Waves

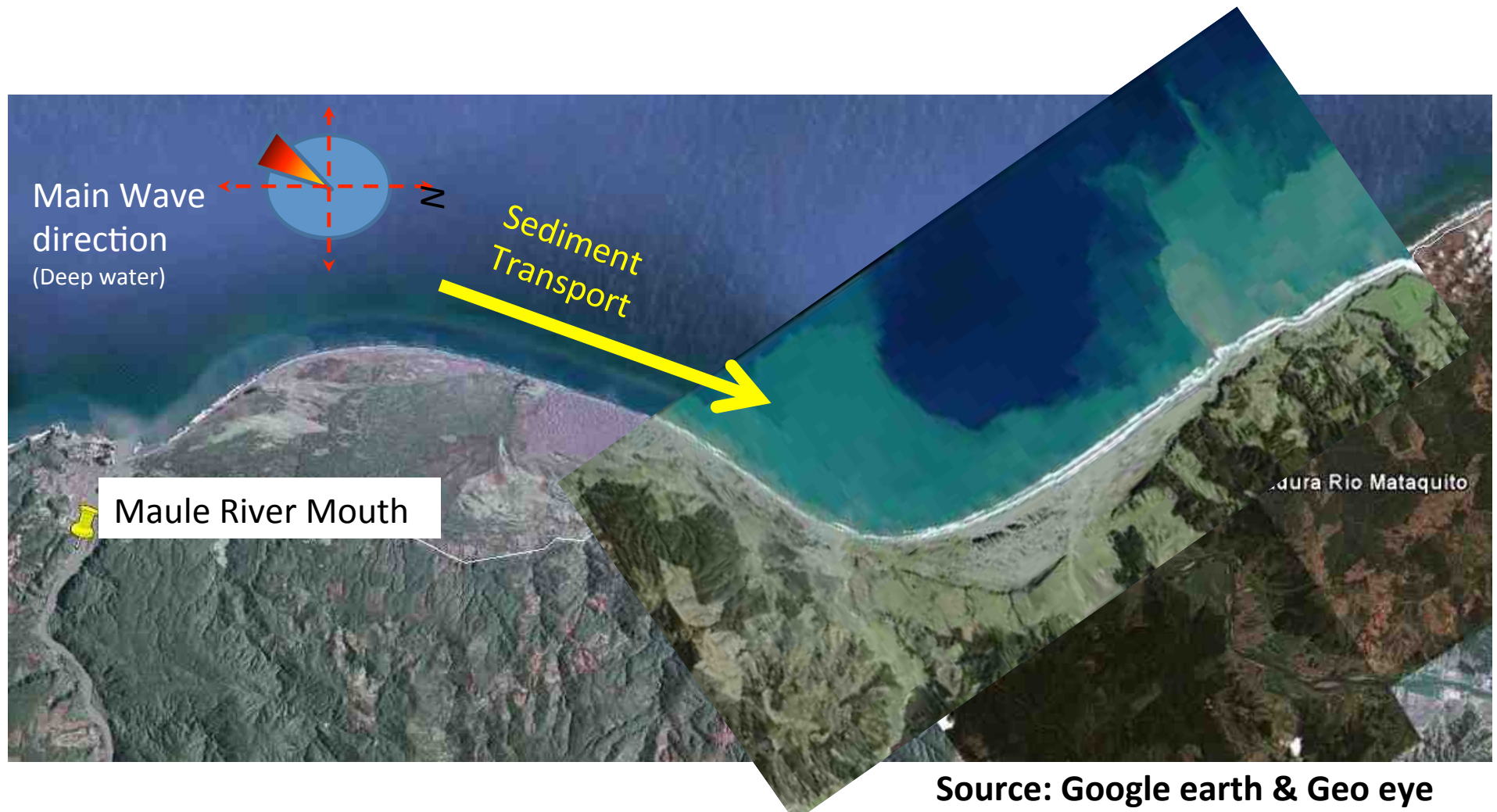
IV. Chile as an International Natural Coastal Laboratory

IV. Natural Coastal Laboratory

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Mataquito River Mouth in the Maule Region



Source: Google earth & Geo eye

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This area was heavily affected by the 2010 Tsunami

- The land subsided (~0.5-1.0m)
- The sand spit was washed away by tsunami waves
- The sand spit rapidly reformed under highly energetic waves (strong wave and aeolian alongshore sand transport)
- Three years of low river discharge

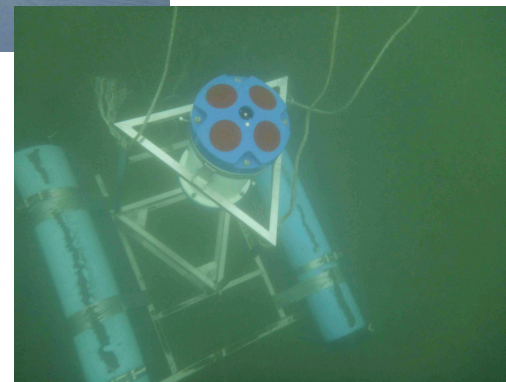
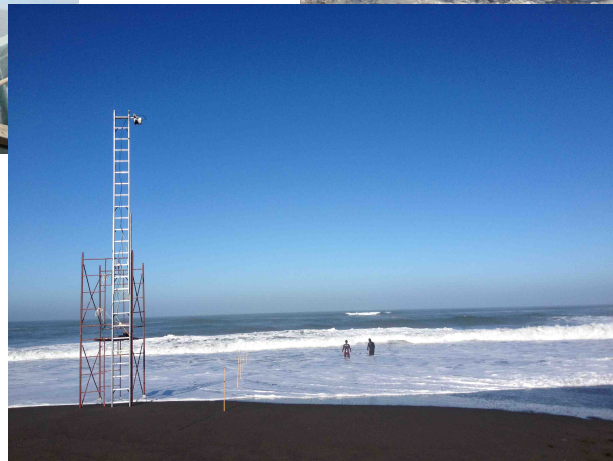
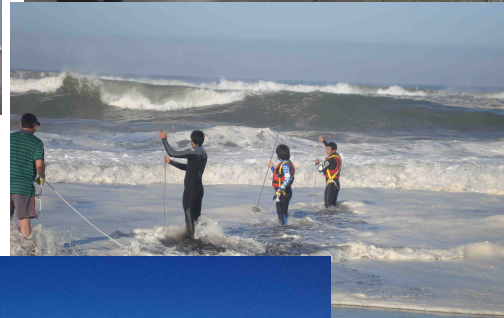
⇒ The sand spits reformed but different than before



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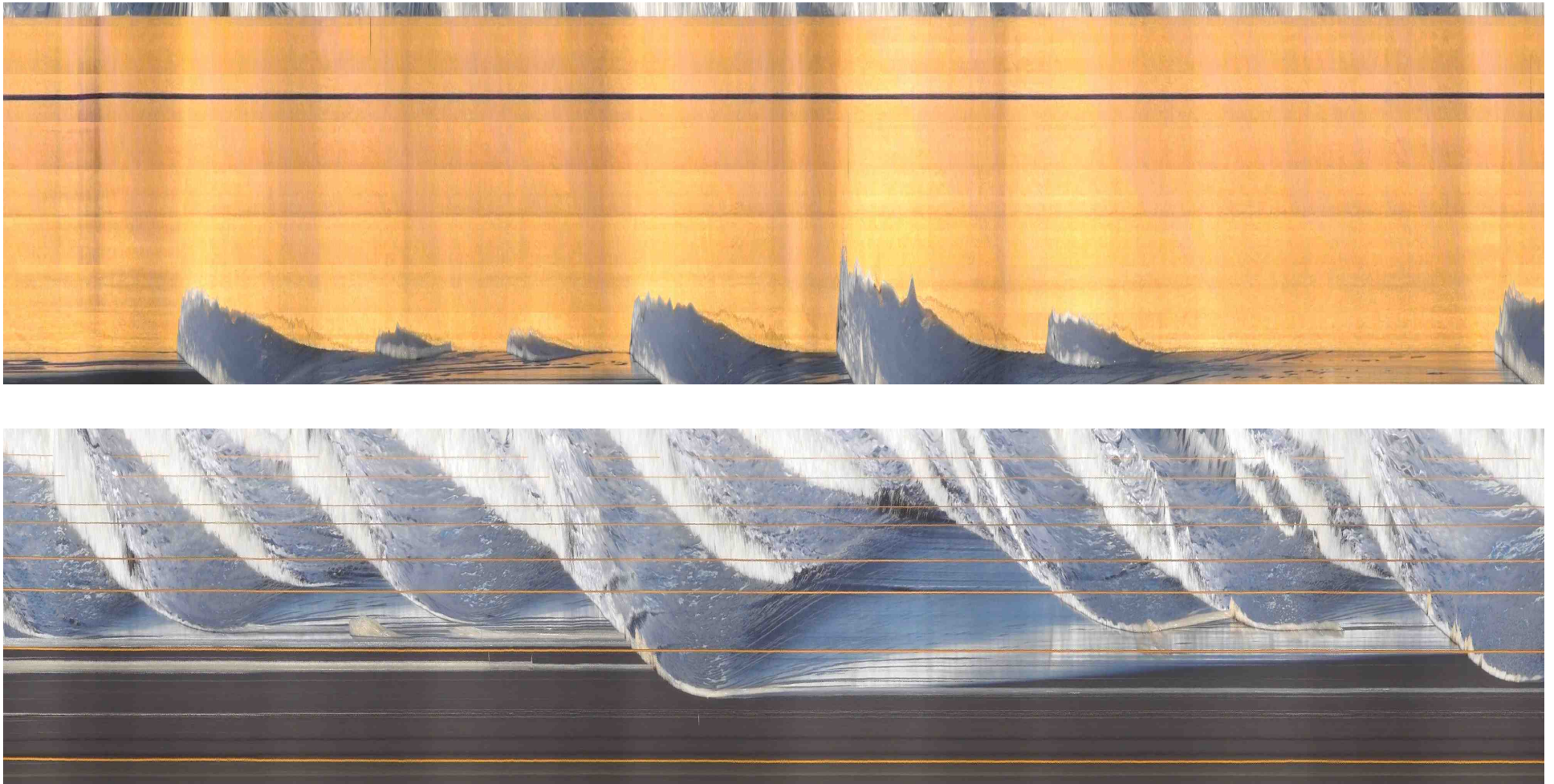
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